H&H Consulting Engineers Pty Ltd (trading as Henry & Hymas) ABN 77 091 243 355 ACN 091 243 355

Address

Suite 2.01, 828 Pacific Highway Gordon New South Wales 2072

Telephone +61 2 9417 8400 Facsimile +61 2 9417 8337

Email email@hhconsult.com.au Web www.henryandhymas.com.au





CIVIL ENGINEERING REPORT

RESIDENTIAL UNIT DEVELOPMENT

17-27 HARDWICKE STREET, RIVERWOOD NSW

Revision 2.0

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Project verification

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	Name	
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Preface

Henry & Hymas has been engaged by NSW Government Land & Housing Corporation to prepare this Civil Engineering Report (The Report) to satisfy civil engineering matters in support of the Development Approval for the proposed Residential Development at 17-27 Hardwicke Street, Riverwood.

The existing site comprises of six residential lots. It is proposed to demolish all the existing residential buildings and develop into a new three-level apartment building with on-site parking, pedestrian footpaths, and landscaped areas.

This Report aims to provide a summary of key civil engineering design elements of the proposed Development Application:

- General site locality, topography and existing characteristics;
- The proposed site works earthworks and site stability;
- Stormwater management;
- Sediment and Erosion.

This Report has been prepared in conjunction with a set of Civil Engineering Drawings which show the general proposed civil and stormwater design for the development. The drawings are available for review in Appendix A of this Report.

The following principles have been adopted as part of the design process:

- Consideration of design intent in relation to functionality, expectations and requirements of the end user.
- Compliance with relevant Council and authority standards and policies.
- Design coordination with the project team.
- A design philosophy sympathetic to the site constraints, environment, terrain, and landform.
- Retention of existing infrastructure where suitable.

The civil and stormwater engineering component of the aforementioned project has been designed in accordance with the following Council codes and policies:

- Georges River Development Control Plan (DCP) 2021.
- Georges River Council Stormwater Management Policy, April 2021.



1. Site overview and Proposed Development

The subject development site is located at 17-27 Hardwicke Street, Riverwood in the Georges River Council Local Government Area (LGA) and covers an area of approximately 3658m². The site is enclosed by Hardwicke Street to the north, and surrounded by existing dwelling houses along the western, eastern and southern boundary lines and further bounded by Hedley Street to the south. Vehicular access to the on-site car park can be achieved via the vehicle crossing and driveway off Hardwicke Street along the eastern site boundary. Pedestrians can access the building via the footpaths and ramps off Hardwicke Street. Refer to Figure 1 below for the locality plan.



Figure 1: Locality plan (Google Maps)

A review of the survey prepared for the subject site shows the site generally falls in an south-westerly direction towards the existing residential lot adjacent to the south-western corner of the site at an average grade of 4.4%. A copy of the feature survey for the development is included in Appendix B of this report. A reduced size image is included below in Figure 2.

The proposed development comprises a three-story residential apartment building with on-grade parking spaces, pedestrian footpaths, and landscaped areas.

An architectural concept has been prepared for the development by Custance Associates Australia. Architectural concept drawings can be found in Appendix C of this report, and excerpt of the site plan is shown below in Figure 3.



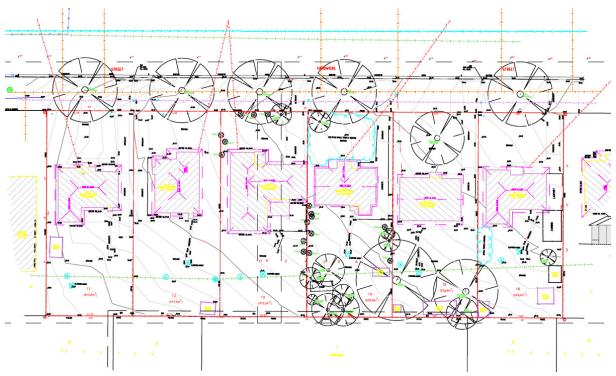
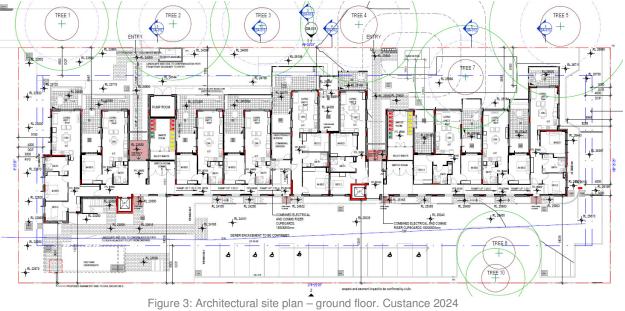


Figure 2: Feature Survey – Northon Survey Partners





2. Site Works

2.1 Bulk Earthworks

Cut and fill earthworks are required to achieve the grades and levels required to construct the proposed apartment building and the associated pedestrian access paths, driveway and car park. The cut and fill earthworks calculations have been undertaken for the overall site. The cut and fill quantities for the site result in a site spoil of material in the order of 969m³. The distribution of cut and fill throughout the site is shown on civil engineering drawings 22T46_DA_BE01, Appendix A. A smaller-scale figure is shown below, Figure 4. Green represents filling and red represents cutting.

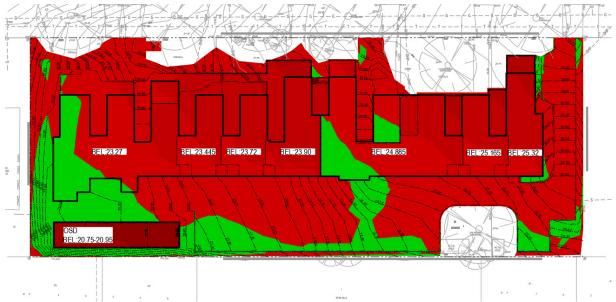


Figure 4: Bulk earthworks cutting and filling Plan. Henry & Hymas 2024



2.2 Site Stability and Retaining Walls

Earthworks batters are provided wherever possible as part of the grading scheme for the development. Earthworks batters will be provided at maximum slopes of 1 in 2 as per geotechnical advice by Section 5.2 of the Geotechnical Report prepared STS Geotechnics dated February 2023, and where short-term construction batters at maximum slopes of 1 in 1 are implemented, these will be protected from erosion by appropriately installed sediment and erosion control measures.

Whilst earthworks batters will be provided wherever possible, retaining walls will be required in some instances where there are significant level changes typically narrow interfaces between existing and proposed levels or fixed site constraints such as Tree Protection Zones (TPZs). The locations and heights of these retaining walls are shown on the engineering drawings located in Appendix A. Retaining walls are required to form compliant grading for the proposed access road, reinforced core-filled blockwork walls are proposed to provide a 'maintenance free' retaining solution for all retaining elements.



3. Stormwater Management

3.1 Existing System

The site generally falls in a south-westerly direction towards the southern site boundary, with the exception of a portion of the site near the north-western corner of the site which fall towards the north-western corner of the site. Based on the existing topography of the site, the stormwater catchment plan under the pre-development scenario can be summarised in Figure 4 below. The red hatch represents the area naturally draining to Hardwicke Street and in the order of 475m². The area shown in green hatch (3183m²) drains to the neighboring sites to the south of the subject development site.



Figure 4: Pre-development stormwater catchment plan. Henry and Hymas 2024

It should be noted that an existing easement for drainage is located along the southern site boundary and extends further towards the south (towards Hedley Street). However, there is no stormwater pipe located within this easement. It is understood under the current scenario that stormwater collected by roof drainage systems is discharged to absorption pits or at ground. Runoff generated by smaller structure or impervious surfaces drains overland towards the southern boundary.

3.2 Proposed Stormwater System

The proposed stormwater management system has been designed to reduce the overall impact of the development on the existing onsite and surrounding stormwater systems and flow regime. The proposed stormwater management system responds to the architectural layout and incorporates the natural topography and site constraints to produce a layout that meets best industry practices and water quality and quantity objectives.

The stormwater management system for the proposed development has been designed to collect all concentrated flows from the proposed impervious areas as well as stormwater runoff generated by pervious areas such as landscaping and earthworks batters. In the event of a total system blockage/failure, site grading is such that overland flow will be directed away from buildings with no impact on habitable areas.

The proposed stormwater system for the development has been designed in accordance and in consideration of the following:



- Institution of Engineers, Australia publication "Australian Rainfall and Runoff" (1987 Edition), Volumes 1 and 2 (AR&R).
- AS 3500.3: National Plumbing and Drainage Code Part 3 Stormwater Drainage.
- Georges River Council Stormwater Management Policy 2021

Another aspect of the stormwater system is to ensure that the design considers water sensitive urban design (WSUD) objectives. The stormwater network has been designed such that it incorporates proprietary Stormwater Quality Improvement Devices (SQIDs) such as Ocean Protect Ocean Guards and Stormfilters as a means of treating stormwater before it leaves the site to ensure there is no adverse impact on the downstream drainage system, Refer Chapter 3.4 and 3.4 for additional details.

As previously noted, the existing drainage easement to the rear (southern boundary) does not contain a stormwater pipe. It should also be noted that Hedley Street itself does not contain inground stormwater system so providing a new pipe in the existing easement would be of no benefit. In ground drainage is located within Hardwicke Street. The location of the existing inground stormwater system in Hardwicke Street is noted on the feature survey and civil engineering drawings included in Appendix B and A respectively. The post developed site catchment is proposed to be discharged to the existing in-ground stormwater system in Hardwick Street. To facilitate the connection a short section of inground stormwater is proposed to be constructed from the property boundary to the nearest pit On-site Stormwater Detention (OSD) system is proposed to be provided to maintain the flow rate of the stormwater discharging to Hardwicke Street in the pre-development scenario. Further details of the OSD system will be discussed in Section 3.2 below. Refer to Figure 5 below for the stormwater catchment plan under the post-development scenario.

It is understood through discussions with Council Engineers that existing residences in Hardwicke Street (number 7 and 9) are currently affected by overland flow/flooding. As such, it is critical to ensure post-developed flow directed to Hardwick Street are equal or less to pre-developed flows. It should be noted that the additional of the piped connection (and new kerb inlet pit in Hardwicke Street) would be of benefit by increasing the inlet capacity of the inground system (reduction in kerb gutter flows) in Hardwicke Street).

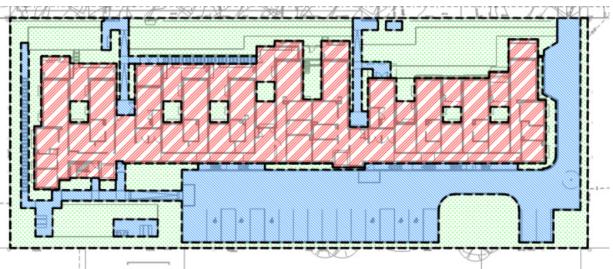


Figure 5: Post-development stormwater catchment plan. Henry and Hymas 2024



3.3 Proposed Stormwater System - On-site Stormwater Detention (OSD)

On-site Stormwater Detention (OSD) will be provided to control the peak stormwater flows from the site by temporarily detaining stormwater from major storms in an underground tank which is then discharged to the downstream drainage system at a controlled rate. In accordance with Georges River Council Stormwater Policy, OSD will be provided to control stormwater runoff from the subject development site such that, for 5-to-100-year ARI events, peak stormwater discharges from the site do not exceed pre-development stormwater discharges.

As mentioned above, the majority of the existing site currently discharges to the southern neighboring sites, only a small portion of the site that is near the north-western corner discharges to Hardwicke Street in the predeveloped scenario.

As per Council Stormwater Management Policy 2021, On-site Stormwater Detention (OSD) system will be required for the site. The required OSD storage requirements and permissible discharge are to be calculated in accordance with the following table.

Site's Impervious Area Percentage upon completion of development (as calculated in accordance with Appendix A7) **	Maximum Permissible Discharge (PSD) L/s/ha	Minimum Site Storage Requirements (SSR) m³/ha
Less than 55%	OSD not required	
55% to less than 65%	182 206	
65% to less than 75%	166	240
75% to less than 85%	152	270
85% or higher	136	295

Table 1: Maximum Permissible Discharge (PSD) and Minimum Site Storage Requirements (SSR)

The site's total impervious area percentage is approximately 65.1% which corresponds to a maximum PSD of 166L/s/ha and minimum SSR of 240m³/ha. The total site area of 3,658m² (0.3658ha) results in a site-specific maximum PSD of 60.72L/s and minimum SSR of 87.79m³.

However, given that the site area discharging to Hardwicke Street increases under the post-development scenario, the OSD is designed to ensure that the post-developed flow to Hardwicke Street are reduced to the pre-developed flow to the street for the 100, 20, 10 and 5 year ARI storm events. The flows discharging to the southern neighboring sites are expected to be significantly reduced post development as it is proposed to drain the entire site towards the north, with the exception of a small portion which is in the order of $65m^2$ and is located at the south-western corner. This small area bypasses the OSD system because it consists of batter down to the boundary levels and is impractical to be drained to the OSD at a higher level. Since this area is significantly smaller than the area draining to the southern neighboring properties under the existing condition, it is expected that the stormwater runoff from this area will infiltrate into the ground without adversely affecting the neighboring properties and is a significant improvement (decrease in run-off) that the current scenario.

The OSD system was designed and the site discharge flows were modelled by using DRAINS modelling software. Refer to the Table 2 below for a summary of the pre and post developed flows for the relevant storm events.

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Detailed drawings and specifications of the proposed below ground OSD tank can be found on the civil engineering drawings 22T4617_DA_C202 included in Appendix A.

The OSD was designed using DRAINS modelling software under design and analysis methods recommend by 2019 edition of Australian Rainfall and Runoff and Council Stormwter policy. Based on the modelling results, the required storage volume for the OSD was calculated to be 160m³. Table 2 below summarises the site discharge flow rates under the pre and post-development conditions for 5 to 100 year ARI events.

ARI	Pre-Development (L/s) North	Pre-Development (L/s) South	Pre-Development (L/s) Total	Post-Development (L/s) From OSD Directed North	Post-Development (L/s) Bypass OSD Overland South	Post-Development (L/s) Total
5	13	59	72	12	0	12
10	16	73	89	13	0	13
20	17	85	102	14	0	14
100	20	115	135	16	0	16

Table 2: Site discharge flow rates for OSD sizing - Pre and post-development conditions

By providing OSD, and from review of Table 1 above summarising the OSD sizing, it is evident that:

- The post-development flows directed to the northwestern corner of the site are less than pre-development flow for 5-to-100-year ARI storm events.
- The post-development flows directed to the southwestern corner of the site are less than pre-development flow for 5-to-100-year ARI storm events.
- The overall post-development flows discharging from the site are less than predevelopment flow for 5-to-100-year ARI storm events.

The OSD will be provided in the form of a proposed below ground tank under the Communal Open Space (COS) near the south-western corner of the site with a volume of 160m³ and a 77mm orifice. The post-developed flow to Hardwicke Street has been restricted to 15L/S (1 in 100 Year ARI storm event) to matches the pre-developed flow to the street by incorporating the OSD system.

It is noted that the proposed design:

- 1. Exceeds Council's SSR requirements for OSD.
- 2. Reduced total site discharge rate in the post developed scenario.
- 3. Matches post developed flows in the 5-year ARI storm event and reduced post developed flows in all other storms directed to Hardwicke Street. It is understood a reduction in post developed flows in the 10-, 20- and 100-Year flow rates would contribute in reducing overland flow in Hardwicke Street to the west of the development.
- 4. Significantly reduces total post developed flow rates. Similarly with point 3, it is understood this would reduce downstream overland flow/flooding.
- 5. Significantly reduces total post developed flow rates for the southern catchments. Similarly with point 3, it is understood this would reduce downstream overland flow/flooding.



3.4 Stormwater Quality

Pollution and contamination dislodged or inherent to and in stormwater and stormwater run-off from urban developments have the potential to damage the ecology and health of local creeks and waterways. As such stormwater quality improvement devices (SQIDs) that aim to minimise pollution during construction and operation of the development have been incorporated into the overall stormwater management design. Further details of the SQIDs that have been proposed for the subject development can be found in Section 3.5: Stormwater Quality Improvement Devices.

Stormwater quality improvement devices (SQIDs) have been sized and measured according to the following guidelines:

• Georges River Council Stormwater Management Policy 2021

The performance of the stormwater quality improvement devices (SQIDs) in mitigating pollution from urban development can be assessed by simulating a post developed pollutant reduction rate for the stormwater system. In order to satisfy the water quality treatment targets in Council's policy, development subject to water quality requirements must achieve a minimum percentage reduction of the post developed average annual loads of pollutants in accordance with the Table e below:

Pollutant	% Post development pollutant reduction targets
Gross Pollutants (GP)	90
Total Suspended Solids (TSS)	80
Total Phosphorous (TP)	40
Total Nitrogen (TN)	40

Table e: Post development pollutant reduction targets

To better determine the conceptual design of the water quality treatment trains and to ensure the treatment trains satisfy the reduction parameters outlined in Table 3, a Model for Urban Stormwater Improvement Conceptualisation (MUSIC) was developed. A schematic of the MUSIC model can be viewed below in Figure 6

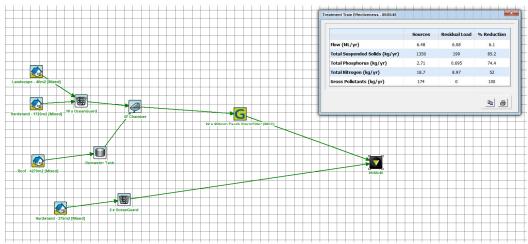


Figure 6: MUSIC modelling results



The resultant reduction in post developed pollutants calculated by the simulation is presented in Table 4 below. With the implementation of the following stormwater quality improvement (SQIDs) devices, the resultant post developed pollutant loads have been reduced below the reduction target for all targeted pollutants.

Pollutant	% Post Development Reduction Target	Resultant % Post Development
Litter / Gross pollutants	90	100
Total Suspended Solids	80	85.2
Total Phosphorous	40	74.4
Total Nitrogen	40	52.0

Table 4: Post development pollutant reduction targets



3.5 Stormwater quality improvement devices (SQIDs)

Primary Treatment - Pit Baskets

As part of an effective treatment train for the site system, selected areas of the development or targeted removal zones (TRZs), will be pre-treated via passive screening pit baskets. To form a site-wide primary treatment system the TRZ for the development encompasses all external areas not beneath roofs and exposed to surface run-off. Target zones, mostly comprising of highly trafficked and hardstand areas that have suitable hydraulics to drain through the basket, are subject to higher instances of pollution and litter and stand most to benefit from effective pre-treatment and have been modelled in MUSIC.

The pit basket proposed to be used is the "200-micron mesh Oceanguard" pit basket filter by Ocean Protect. Pit baskets to be fitted with the "Oceanguard" bit basket are noted on drawing 22T46_DA_C200. A detailed of the pit baskets is provide on civil engineering drawings 22T46_DA_C201.

The maintenance of the pit baskets is important to ensure the effective removal of pollutants. As such, a maintenance schedule will be required to be detailed at the Construction Certificate stage. For this submission we have provided the generic maintenance schedule as prepared by Ocean Protect, refer to Appendix D.

Secondary treatment - Filter cartridges

To meet the water quality requirements, and to achieve a successful implement a treatment train approach, a media filtration chamber is located at the end of the major stormwater drainage line. The media filtration (PSorb Stormfilter cartridges by Ocean Protect) targets the removal of gross pollutants, suspended solids, targeted nutrients and hydrocarbons within the stormwater run-off generated by the developed site areas and is proposed to be used in this development.

Media filtration will be incorporated into the stormwater system by the addition of an offline Stormfilter chamber housed with the OSD tank. The Stormfilter chamber is proposed to house three (3) x 690mm PSorb filter cartridges. Internal site stormwater will be directed to the filter chamber as shown on the civil engineering plan 22T46 DA C202.

The maintenance of the filter cartridges is important to ensure the effective removal of pollutants. As such, a maintenance schedule will be required to be detailed at the Construction Certificate stage. For this submission we have provided the generic maintenance schedule as prepared by Ocean Protect, refer Appendix D.



4. Sediment and Erosion Controls

During construction, appropriate sediment and erosion control measures need to be implemented to ensure that downstream receiving waters are not adversely impacted as a result of construction activities. The engineering drawings 22T46_DA_SE01-SE02 by Henry & Hymas outline appropriately designed and detailed measures to mitigate against this risk. These measures have been designed in accordance with the requirements of the publication "Landcom – Managing Urban Stormwater - Soils and Construction, Volume 1, 4th Edition March 2004" and Georges River Council's requirements.

The below table is provided to indicate all factors accounted for in the design of the sediment basin.

Constraint	Value	(Source)*
Rainfall Erosivity (R-factor)	2800	Appendix B - Map 10
Length/Slope Gradient Factor, LS	0.97	Appendix A - Table A1
Soil Erodibility (K-factor)	0.038	Appendix C -Table 19
Erosion Control Practice Factor (P-Factor)	1.3	Appendix A - Table A2
Cover Factor (C-Factor)	1.0 (During earthworks)	Appendix A - Figure A5
Calculated Soil Loss, A (RUSLE equation)	134.2	A = R K LS P C
Soil Hydrologic Group	С	Appendix C Table 19
75 th Percentile 5-day Rainfall Event	19.4mm (Bankstown)	Table 6.3a
Volumetric Runoff Coefficient, Cv	0.39	(App. F Table F2)
Catchment Area, A	0.3658 ha	

3. Table 3.1: Sediment Basin design parameters

Settling Zone volumes and Sediment Zone volumes were calculated using the below calculations

Volume	Formula
Settling Zone Volume	10 Cv A R
Sediment Zone Volume	0.17A(R K LS P C)/1.3 (if <50% of settling zone, nominate 50% of settling zone)
Total Sediment Basin Volume Required :	Settling Zone + Sediment Zone

4. Table 3.2: Sediment Basin calculations



Below is a table indicating the total sediment basin volumes required for the site.

	Area (ha)	Settling zone (m³)	Sediment zone (m³)	Total Sediment Basin Vol. (m³)
Subject Site	0.3658	27.7	17.5	45.2

6. Table 3.3: Sediment Basin volumes

5. Conclusion

In general, the engineering objectives of civil design and stormwater management elements mentioned above are to create a system that is based on the architectural layout and incorporates the natural topography and site constraints to produce a cost-effective and appropriate drainage system that meets best industry practices and governing water quality and quantity objectives.

We trust the information provided in this report satisfies matters relating to bulk earthworks, site stability and stormwater matters such as surrounding drainage, On-site Stormwater Detention, and water quality.



6. Appendices

Appendix A: Civil Engineering Drawings by Henry & Hymas Engineers

Appendix B: Site Survey

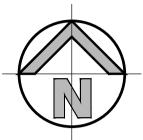
Appendix C: Architectural concept drawings – Site plan

Appendix D: Maintenance Manual



Appendix A: Civil Engineering Drawings by Henry & Hymas Engineers

PROPOSED RESIDENTIAL DEVELOPMENT 17-27 HARDWICKE ST, RIVERWOOD, NSW CIVIL WORKS





LOCALITY SKETCH

SCALE: N.T.S.

DRAWING SCHEDULE		
22T46_DA_C000	COVER SHEET, DRAWINGS SCHEDULE AND LOCALITY SKETCH	
22T46_DA_C010	NOTES	
22T46_DA_C100	GENERAL ARRANGEMENT PLAN	
22T46_DA_C101	DETAIL PLAN - SHEET 1 OF 2	
22T46_DA_C102	DETAIL PLAN - SHEET 2 OF 2	
22T46_DA_C110	TYPICAL KERB AND CONCRETE DETAILS	
22T46_DA_C160	STANDARD DRAWINGS - CROSSOVER, FOOTPATH	
22T46_DA_C200	STORMWATER MISCELLANEOUS DETAILS AND PIT LID SCHEDULE	
22T46_DA_C201	STORMWATER MISCELLANEOUS DETAILS	
22T46_DA_C202	OSD TANK PLAN DETAILS AND SECTIONS	
22T46_DA_C250	PRE-DEVELOPMENT STORMWATER CATCHMENT PLAN	
22T46_DA_C251	POST-DEVELOPMENT STORMWATER CATCHMENT PLAN	
22T46_DA_C310	RETAINING WALL DETAILS	
22T46_DA_SE01	SEDIMENT AND EROSION CONTROL PLAN	
22T46_DA_SE02	SEDIMENT AND EROSION CONTROL DETAILS	
22T46_DA_EX01	EXTERNAL WORKS PLAN - SHEET 1 OF 4	
22T46_DA_EX02	EXTERNAL WORKS PLAN - SHEET 2 OF 4	
22T46_DA_EX03	EXTERNAL WORKS PLAN - SHEET 3 OF 4	
22T46_DA_EX04	EXTERNAL WORKS PLAN - SHEET 4 OF 4	
22T46_DA_BE01	BULK EARTHWORKS CUT AND FILL PLAN	
22T46_DA_BE02	BULK EARTHWORKS CUTAND FILL DEPTH PLAN	
22T46_DA_EX10	EXTERNAL FOOTPATH LONG SECTION	
22T46_DA_EX11	EXTERNAL FOOTPATH CROSS SECTIONS	
22T46_DA_EX12	EXTERNAL FOOTPATH RAMP SECTIONS	

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GENERAL NOTES:

- 1. ALL WORK TO BE CARRIED OUT IN ACCORDANCE WITH GEORGES RIVER COUNCIL SPECIFICATION. CONTRACTOR TO OBTAIN AND RETAIN A COPY ON SITE DURING THE COURSE OF THE WORKS.
- 2. ALL NEW WORKS ARE TO MAKE A SMOOTH JUNCTION WITH EXISTING CONDITIONS AND MARRY IN A 'WORKMANLIKE' MANNER.
- 3. THE CONTRACTOR IS TO VERIFY THE LOCATION OF ALL SERVICES WITH EACH RELEVANT AUTHORITY. ANY DAMAGE TO SERVICES SHALL BE RECTIFIED BY THE CONTRACTOR OR THE RELEVANT AUTHORITY AT THE CONTRACTOR'S EXPENSE. SERVICES SHOWN ON THESE PLANS ARE ONLY THOSE EVIDENT AT THE TIME OF SURVEY OR AS DETERMINED FROM SERVICE DIAGRAMS. H & H CONSULTING ENGINEERS PTY. LTD CANNOT GUARANTEE THE INFORMATION SHOWN NOR ACCEPT ANY RESPONSIBILITY FOR INACCURACIES OR INCOMPLETE DATA.
- 4. SERVICES & ACCESSES TO THE EXISTING PROPERTIES ARE TO BE MAINTAINED IN WORKING ORDER AT ALL TIMES DURING CONSTRUCTION.
- ADJUST EXISTING SERVICE COVERS TO SUIT NEW FINISHED LEVELS TO RELEVANT AUTHORITY REQUIREMENTS WHERE NECESSARY.
- REINSTATE AND STABILISE ALL DISTURBED LANDSCAPED AREAS.
- MINIMUM GRADE OF SUBSOIL SHALL BE 0.5% (1:200) FALL TO OUTLETS.
- ALL TEMPORARY SEDIMENT AND EROSION CONTROL DEVICES ARE TO BE CONSTRUCTED PLACED AND MAINTAINED IN ACCORDANCE WITH THE TECHNICAL SPECIFICATIONS, EROSION AND SEDIMENTATION CONTROL PLAN AND CAMDEN COUNCIL'S REQUIREMENTS WHERE APPLICABLE.
- CONTRACTOR TO CHECK AND CONFIRM SITE DRAINAGE CONNECTIONS ACROSS THE VERGE PRIOR TO COMMENCEMENT OF SITE DRAINAGE WORKS.
- 10. PROPERTIES AFFECTED BY THE WORKS ARE TO BE NOTIFIED IN ADVANCE WHERE DISRUPTION TO EXISTING ACCESS IS LIKELY

SURVEY NOTES

- 1. THE EXISTING SITE CONDITIONS SHOWN ON THE FOLLOWING DRAWINGS HAVE BEEN INVESTIGATED BY THE SURVEYOR SPECIFIED IN THE TITLE BLOCK
- 2. THE INFORMATION IS SHOWN TO PROVIDE A BASIS FOR DESIGN HENRY AND HYMAS PTY. LTD. DOES NOT GUARANTEE THE ACCURACY OR COMPLETENESS OF THE SURVEY BASE OR ITS SUITABILITY AS A BASIS FOR CONSTRUCTION DRAWINGS.
- SHOULD DISCREPANCIES BE ENCOUNTERED DURING CONSTRUCTION BETWEEN THE SURVEY DATA AND ACTUAL FIELD DATA, CONTACT HENRY AND HYMAS PTY. LTD. THE FOLLOWING NOTES HAVE BEEN TAKEN DIRECTLY FROM ORIGINAL SURVEY DOCUMENTS.

EXISTING SERVICES & FEATURES

- THE CONTRACTOR SHALL ALLOW FOR THE CAPPING OFF, EXCAVATION AND REMOVAL (IF REQUIRED) OF ALL EXISTING SERVICES IN AREAS AFFECTED BY WORKS WITHIN THE CONTRACT AREA OR AS SHOWN ON THE DRAWINGS UNLESS DIRECTED OTHERWISE BY THE SUPERINTENDENT.
- THE CONTRACTOR SHALL ENSURE THAT AT ALL TIMES SERVICES TO ALL BUILDINGS NOT AFFECTED BY THE WORKS ARE NOT DISRUPTED.
- PRIOR TO COMMENCEMENT OF ANY WORKS THE CONTRACTOR SHALL GAIN APPROVAL OF HIS PROGRAM FOR THE RELOCATION/ CONSTRUCTION OF TEMPORARY SERVICES.
- CONTRACTOR SHALL CONSTRUCT TEMPORARY SERVICES TO MAINTAIN SUPPLY TO EXISTING BUILDING REMAINING IN OPERATION DURING WORKS TO THE SATISFACTION AND APPROVAL OF THE SUPERINTENDENT. ONCE DIVERSION IS COMPLETE AND COMMISSIONED, THE CONTRACTOR SHALL REMOVE ALL SUCH TEMPORARY SERVICES AND MAKE GOOD TO THE SATISFACTION OF THE SUPERINTENDENT.
- INTERRUPTION TO SUPPLY OF EXISTING SERVICES SHALL BE DONE SO AS NOT TO CAUSE ANY INCONVENIENCE TO THE PRINCIPAL. CONTRACTOR TO GAIN APPROVAL FROM THE SUPERINTENDENT FOR TIME OF INTERRUPTION.
- EXISTING SERVICES, BUILDINGS, EXTERNAL STRUCTURES AND TREES SHOWN ON THESE DRAWINGS ARE EXISTING FEATURES PRIOR TO ANY DEMOLITION WORKS.
- EXISTING SERVICES UNLESS SHOWN ON SURVEY PLAN HAVE BEEN PLOTTED FROM SERVICES SEARCH PLANS AND AS SUCH THEIR ACCURACY CANNOT BE GUARANTEED. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO COMPLETE A 'DIAL BEFORE YOU DIG' SEARCH AND TO ESTABLISH THE LOCATION AND LEVEL OF ALL EXISTING SERVICES PRIOR TO THE COMMENCEMENT OF ANY WORK. ANY DISCREPANCIES SHALL BE REPORTED TO THE SUPERINTENDENT. CLEARANCES SHALL BE OBTAINED FROM THE RELEVANT SERVICE
- ALL BRANCH GAS AND WATER SERVICES UNDER DRIVEWAYS AND BRICK PAVING SHALL BE LOCATED IN Ø80 uPVC SEWER GRADE CONDUITS EXTENDING A MINIMUM OF 500mm BEYOND EDGE OF PAVING.

SITEWORKS NOTES

PRIOR TO THE COMMENCEMENT OF WORK.

- 1. DATUM : A.H.D.
- 2. ORIGIN OF LEVELS: REFER TO BENCH OR STATE SURVEY MARKS WHERE SHOWN ON PLAN.
- CONTRACTOR MUST VERIFY ALL DIMENSIONS AND EXISTING LEVELS ON SITE
- 4. ALL WORKS TO BE UNDERTAKEN IN ACCORDANCE WITH THE DETAILS SHOWN ON THE DRAWINGS & THE DIRECTIONS OF THE SUPERINTENDENT.
- EXISTING SERVICES UNLESS SHOWN ON THE SURVEY PLAN HAVE BEEN PLOTTED FROM SERVICES SEARCH PLANS AND AS SUCH THEIR ACCURACY CANNOT BE GUARANTEED. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ESTABLISH THE LOCATION AND LEVEL OF ALL EXISTING SERVICES PRIOR TO THE COMMENCEMENT OF ANY WORK. ANY DISCREPANCIES SHALL BE REPORTED TO THE SUPERINTENDENT. CLEARANCES SHALL BE OBTAINED FROM THE RELEVANT SERVICE AUTHORITY.
- WHERE NEW WORKS ABUT EXISTING THE CONTRACTOR SHALL ENSURE THAT A SMOOTH EVEN PROFILE, FREE FROM ABRUPT CHANGES IS ACHIEVED.
- THE CONTRACTOR SHALL ARRANGE ALL SURVEY SETOUT TO BE CARRIED OUT BY A REGISTERED SURVEYOR.

MECHANICAL EXCAVATION IS TO BE UNDERTAKEN OVER TELSTRA OR

- 8. CARE IS TO BE TAKEN WHEN EXCAVATING NEAR EXISTING SERVICES. NO
- 9. CONTRACTOR TO OBTAIN AUTHORITY APPROVALS WHERE APPLICABLE

ELECTRICAL SERVICES. HAND EXCAVATE IN THESE AREAS.

- 10. MAKE SMOOTH TRANSITION TO EXISTING SURFACES AND MAKE GOOD.
- 11. THESE PLANS SHALL BE READ IN CONJUNCTION WITH APPROVED LANDSCAPE, ARCHITECTURAL, STRUCTURAL, HYDRAULIC AND MECHANICAL DRAWINGS AND SPECIFICATIONS
- 12. OR WRITTEN INSTRUCTIONS THAT MAY BE ISSUED RELATING
- 13. TO DEVELOPMENT AT THE SITE.
- 14. TRENCHES THROUGH EXISTING ROAD AND CONCRETE PAVEMENTS SHALL BE SAWCUT TO FULL DEPTH OF CONCRETE AND A MINIMUM OF 50mm IN BITUMINOUS PAVING.
- 15. ALL BRANCH GAS AND WATER SERVICES UNDER DRIVEWAYS AND BRICK PAVING SHALL BE LOCATED IN Ø80 uPVC SEWER GRADE CONDUITS EXTENDING A MINIMUM OF 500mm BEYOND EDGE OF PAVING.
- 16. GRADES TO PAVEMENTS TO BE AS IMPLIED BY RL'S ON PLAN. GRADE EVENLY BETWEEN NOMINATED RL'S. AREAS EXHIBITING PONDING GREATER THAN 5mm DEPTH WILL NOT BE ACCEPTED UNLESS IN A DESIGNATED SAG
- 17. ALL COVERS AND GRATES ETC TO EXISTING SERVICE UTILITIES ARE TO BE ADJUSTED TO SUIT NEW FINISHED SURFACE LEVELS WHERE APPLICABLE.

COMPACTION REQUIREMENTS

- 1. PREPARATION FOR PAVEMENT: CLEAR SITE, STRIP TOP-SOIL, CUT AND FILL AND PREPARATIONS OF SUB-GRADE SHALL BE AS DESCRIBED IN "SUBGRADE PREPARATION - SITEWORKS".
- SUB-GRADE SHALL BE COMPACTED TO 98% STANDARD MAXIMUM DRY DENSITY RATIO AT OPTIMUM MOISTURE CONTENT ± 2% IN ACCORDANCE WITH AS 1289 5.1.1., TOP 300MM TO 100% SDD.
- 3. FOR A SAND SUBGRADE, COMPACTION SHALL BE TO MIN.90% DENSITY INDEX AS PER AS1289 E6.1
- 4. LOWER BASE COURSE SHALL BE CONSTRUCTED FROM CRUSHED ROCK COMPACTED TO 98% MODIFIED MAXIMUM DRY DENSITY RATIO AT OPTIMUM MOISTURE CONTENT ± 2% IN ACCORDANCE WITH AS 1289 5.1.1. OF THICKNESS NOTED ON DRAWINGS.
- BASE COURSE SHALL BE CONSTRUCTED FROM FINE CRUSHED ROCK COMPACTED TO 98% MODIFIED MAXIMUM DRY DENSITY RATIO AT OPTIMUM MOISTURE CONTENT ± 2% IN ACCORDANCE WITH AS 1289 5.1.1 OF THICKNESS NOTED ON DRAWINGS.
- WEARING SURFACE SHALL BE ASPHALTIC CONCRETE TO STANDARD SPECIFICATION

APPROVED N.A.T.A. REGISTERED LABORATORY

7. TESTING OF THE SUBGRADE AND PAVEMENT LAYERS SHALL BE CARRIED OUT BY

BULK EARTHWORKS/ SUBGRADE PREPARATION - SITEWORKS

- REFER TO GEOTECHNICAL INVESTIGATION REPORT OR INFORMATION RELATING TO EXISTING GROUND CONDITIONS, SITE TREATMENT AND SUPERVISION. REPORT BY STS GEOTECHNICS REFERENCE 23/0234.
- THE LOCATIONS OF UNDERGROUND SERVICES SHOWN ON THESE DRAWINGS HAVE BEEN PLOTTED FROM SURVEY AND AUTHORITY INFORMATION. THE SERVICE INFORMATION HAS BEEN PREPARED ONLY TO SHOW THE APPROXIMATE POSITIONS OF ANY KNOWN SERVICES AND MAY NOT BE AS CONSTRUCTED OR ACCURATE.
- HENRY AND HYMAS PTY LTD CAN NOT GUARANTEE THAT THE SERVICES INFORMATION SHOWN ON THESE DRAWINGS, ACCURATELY INDICATES THE PRESENCE OR ABSENCE OF SERVICES OR THEIR LOCATION AND WILL ACCEPT NO LIABILITY FOR INACCURACIES IN THE SERVICES INFORMATION SHOWN ARISING FROM ANY CAUSE WHATSOEVER. CONTRACTORS ARE TO CONTACT THE RELEVANT SERVICE AUTHORITY PRIOR TO COMMENCEMENT OF EXCAVATION. FOR COMMENCEMENT OF WORKS ON SITE, SEARCH RESULTS ARE TO BE
- ALL SERVICES ARE TO BE LOCATED AND CUT OFF PRIOR TO THE COMMENCEMENT OF EXCAVATION AND FILLING OPERATIONS.

KEPT ON SITE AT ALL TIMES.

- ALL TOP SOIL, ORGANIC MATTER AND FILL MATERIAL SHALL BE REMOVED FROM ALL AREAS UNDER BUILDING AND CARPARK LOCATIONS TO THE SATISFACTION OF THE GEOTECHNICAL ENGINEER. AREAS TO BE FULLY STRIPPED OF EXISTING FILL AND DARK BROWN BLACK UPPER ORGANIC ALLUVIUM, OR OBVIOUS UNSUITABLE MATERIAL
- EXCAVATE TO ACHIEVE SUBGRADE LEVELS WHERE NECESSARY.
- THE EXPOSED SUBGRADE AFTER STRIPPING AND/ OR EXCAVATION IS TO BE PROOF ROLLED TO GEOTECHNICAL ENGINEERS SPECIFICATIONS UNDER THE SUPERVISION OF AN EXPERIENCED GEOTECHNICAL ENGINEER OR AN EXPERIENCED CIVIL ENGINEER. ANY AREAS ON THE SUBGRADE EXHIBITING EXCESSIVE DEFLECTION / MOVEMENT UNDER ROLLER TO BE EXCAVATED TO A MIN. DEPTH OF 0.5m AND REPLACED WITH APPROVED GRANULAR MATERIAL COMPACTED IN 250mm LOOSE LAYERS OR AS DIRECTED BY THE GEOTECHNICAL ENGINEER.
- ENGINEERED FILL FOR REPLACEMENT OF SOFT OR HEAVING AREAS OR FOR BULK FILLING TO COMPRISE ESSENTIALLY OF GRANULAR MATERIALS (EG. EXCAVATED SHALE), WITH A PARTICLE SIZE NOT GREATER THAN 75mm DIAMETER. ENGINEERED FILL TO BE PLACED IN LAYERS NOT EXCEEDING 250mm LOOSE THICKNESS AND COMPACTED TO 100% OF STANDARD MAXIMUM DRY DENSITY (SMDD) WITHIN ± 2% OF OPTIMUM MOISTURE CONTENT (OMC).
- IMPORTED FILLING (IF REQUIRED) IS TO BE TO THE APPROVAL OF THE GEOTECHNICAL ENGINEER. THE CONTRACTOR IS TO NOMINATE THE SOURCE AND PROVIDE A SAMPLE FOR APPROVAL PRIOR TO IMPORTATION AND PLACEMENT ON SITE.
- 10. ALL FILL MATERIAL SHALL BE FROM A SOURCE APPROVED BY THE SUPERINTENDENT AND SHALL COMPLY WITH THE FOLLOWING. FREE FORM ORGANIC AND PERISHABLE MATTER MAXIMUM PARTICLE SIZE = 75mm MAXIMUM PLASTICITY INDEX = 15% MIN CBR 5%
 - ALL IMPORTED FILL MATERIAL SHALL BE IN ACCORDANCE WITH SPECIFICATIONS FROM GEOTECH REPORT.
- 11. ALL EARTHWORKS SHALL BE CARRIED OUT IN ACCORDANCE WITH GEOTECH REPORT.
- IN-SITU DENSITY TESTING AND SUPERVISION MUST BE CARRIED OUT IN ACCORDANCE WITH THE RECOMMENDATIONS CONTAINTED WITHIN GEOTECH REPORT.

SUBGRADE PREPARATION - SITEWORKS.

- THE EXISTING SURFACE IS TO BE STRIPPED OF ANY PAVEMENTS, TOPSOIL OR OBVIOUS UNSUITABLE MATERIAL
- EXCAVATE TO ACHIEVE SUBGRADE LEVELS WHERE NECESSARY.
- THE EXPOSED SUBGRADE AFTER STRIPPING AND/ OR EXCAVATION IS TO BE PROOF ROLLED USING NOT FEWER THAN 5 PASSES OF A MINIMUM 8 TONNE DEAD WEIGHT STEEL SMOOTH-DRUM ROLLER UNDER THE SUPERVISION OF AN EXPERIENCED GEOTECHNICAL ENGINEER OR AN EXPERIENCED CIVIL ENGINEER. ANY AREAS ON THE SUBGRADE EXHIBITING EXCESSIVE DEFLECTION / MOVEMENT UNDER ROLLER TO BE EXCAVATED TO A MIN. DEPTH OF 0.5m AND REPLACED WITH APPROVED GRANULAR MATERIAL COMPACTED IN 250mm LOOSE LAYERS OR AS DIRECTED BY THE GEOTECHNICAL ENGINEER.
- ENGINEERED FILL FOR REPLACEMENT OF SOFT OR HEAVING AREAS OR FOR BULK FILLING TO COMPRISE ESSENTIALLY OF GRANULAR MATERIALS (EG. EXCAVATED SHALE), WITH A PARTICLE SIZE NOT GREATER THAN 75mm DIAMETER. ENGINEERED FILL TO BE PLACED IN LAYERS NOT EXCEEDING 250mm LOOSE THICKNESS AND COMPACTED TO BETWEEN 98% AND 102% OF STANDARD MAXIMUM DRY DENSITY (SMDD) WITHIN ± 2% OF OPTIMUM MOISTURE CONTENT (OMC).
- IMPORTED FILLING (IF REQUIRED) IS TO BE TO THE APPROVAL OF THE GEOTECHNICAL ENGINEER. THE CONTRACTOR IS TO NOMINATE THE SOURCE AND PROVIDE A SAMPLE FOR APPROVAL PRIOR TO IMPORTATION AND PLACEMENT ON SITE.
- ALL FILL MATERIAL SHALL BE FROM A SOURCE APPROVED BY THE SUPERINTENDENT AND SHALL COMPLY WITH THE FOLLOWING. FREE FORM ORGANIC AND PERISHABLE MATTER MAXIMUM PARTICLE SIZE = 75mm MAXIMUM PLASTICITY INDEX = 15%

FLEXIBLE PAVEMENT NOTES:

BASE, SUB BASE AND SELECT MATERIAL

- PAVEMENT "BASE" (I.E. THE HIGHEST COURSE OF THE PAVEMENT BELOW THE SURFACING) SHALL BE CONSTRUCTED OF FINE CRUSHED ROCK (EXCEPT CONCRETE PAVEMENT. WHERE THE CONCRETE IS THE BASE) PAVEMENT "SUB-BASE" (I.E. THE INTERMEDIATE OR LOWER COURSE OF THE PAVEMENT BELOW THE BASE) SHALL BE CONSTRUCTED OF CRUSHED ROCK OR NATURAL GRAVELS. PAVEMENT "SELECT MATERIAL LAYER" (I.E. THE LOWER COURSE OF THE
- PAVEMENT BELOW THE SUB-BASE) SHALL BE CONSTRUCTED OF CRUSHED ROCK, NATURAL GRAVELS OR SUITABLE SOILS. THE SUBGRADE SURFACE SHALL BE THE SURFACE THAT UNDERLIES EITHER:

• THE SELECT MATERIAL LAYER WHEN SELECT MATERIAL LAYER IS PRESENT, OR

- THE SUB-BASE WHEN SELECT MATERIAL LAYER IS ABSENT FROM THE PAVEMENT CONSTRUCTION, OR
- THE BASE WHEN BOTH SELECT MATERIAL LAYER AND SUB-BASE ARE ABSENT FROM THE PAVEMENT CONSTRUCTION

PRIOR TO THE DELIVERY OF ANY MATERIAL TO THE SITE. THE SOURCE OF ALL MATERIALS AND CERTIFICATES THAT THE MATERIAL SATISFIES THE SPECIFIED REQUIREMENTS SHALL BE PROVIDED FOR APPROVAL. ADDITIONALLY FOR EACH MATERIAL SOURCE, COMPLIANCE WITH THE APPROVED QUALITY ASSURANCE PROGRAM FOR INDIVIDUAL MATERIALS SHALL BE PROVIDED TO THE SUPERINTENDENT AS REQUIRED IN THE QUALITY SYSTEM REQUIREMENTS. TESTING OF PAVEMENT MATERIALS WILL NORMALLY BE PERFORMED ON SAMPLES TAKEN AT THE SOURCE PRIOR TO DELIVERY TO THE SITE AND IN THEIR FINAL CONDITIONS AFTER PLACEMENT AND COMPACTION IN THE PAVEMENT, HOWEVER, THE PROPERTIES SPECIFIED AND FINAL ACCEPTANCE ARE APPLICABLE TO THE MATERIALS IN THEIR FINAL CONDITION IN THE PAVEMENT. FINAL ACCEPTANCE WILL BE CONDITIONAL ON NO SIGNIFICANT CHANGE IN PROPERTIES DUE TO SEGREGATION OR CONTAMINATION DURING SUBSEQUENT PAVEMENT WORKS.

FINE CRUSHED ROCK BASE MATERIAL

FINE CRUSHED ROCK SHALL CONFORM TO THE REQUIREMENTS OF CLASS DGB20 MATERIAL AS SPECIFIED IN RMS SPECIFICATION 3051 AND SHALL BE HARD, DURABLE STONE FREE OF CLAY LUMPS, ORGANIC MATTER AND OBJECTIONABLE QUANTITIES OF DELETERIOUS SUBSTANCES. THE MATERIAL MAY BE CRUSHER RUN OR SCREENED AND RECOMBINED. ALL THE MATERIAL REQUIREMENTS APPLY BOTH PRIOR TO AND AFTER PLACEMENT IN THE PAVEMENT.

SUB-BASE MATERIALS

SUB-BASE MATERIALS SHALL BE CRUSHED ROCK SUB-BASE OR SUITABLE NATURAL GRAVELS AND CONFORM TO THE REQUIREMENTS OF CLASS DGS40 OR DGS20 OF RMS SPECIFICATION 3051. STONE SHALL BE HARD, DURABLE AND THE MATERIALS SHALL BE FREE OF CLAY LUMPS, ORGANIC MATTER AND OBJECTIONABLE QUANTITIES OF DELETERIOUS SUBSTANCES. ALL MATERIAL REQUIREMENTS WILL APPLY BOTH PRIOR TO AND AFTER PLACEMENT IN THE PAVEMENT.

KERBING NOTES:

INCLUDES ALL KERBS, GUTTERS, DISH DRAINS, CROSSING AND EDGES.

- 1. ALL KERBS, GUTTERS, DISH DRAINS AND CROSSING TO BE CONSTRUCTED ON MINIMUM 75mm GRANULAR BASE COURSE COMPACTED TO MINIMUM 98% MODIFIED MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS 1289 5.2.1.
- 2. EXPANSION JOINTS (EJ) TO BE FORMED FROM 10mm COMPRESSIBLE CORK FILTER BOARD FOR THE FULL DEPTH OF THE SECTION AND CUT TO PROFILE. EXPANSION JOINTS TO BE LOCATED AT DRAINAGE PITS, ON TANGENT POINTS OF CURVES AND ELSEWHERE AT 12m CENTRES EXCEPT FOR INTEGRAL KERBS WHERE THE EXPANSION JOINTS ARE TO MATCH THE JOINT LOCATION IN
- 3. WEAKENED PLANE JOINTS TO BE MIN 3mm WIDE AND LOCATED AT 3m CENTRES EXCEPT FOR INTEGRAL KERBS WHERE WEAKENED PLANE JOINTS ARE TO MATCH THE JOINT LOCATIONS IN SLABS.
- 4. BROAMED FINISHED TO ALL RAMPED AND VEHICULAR CROSSINGS, ALL OTHER KERBING OR DISH DRAINS TO BE STEEL FLOAT FINISHED.
- IN THE REPLACEMENT OF KERBS

SHOWN.

EXISTING ROAD PAVEMENT IS TO BE SAWCUT 900mm FROM LIP OF GUTTER. UPON COMPLETION OF NEW KERBS. NEW BASE COURSE AND SURFACE IS TO BE LAID 900mm WIDE TO MATCH EXISTING MATERIALS AND THICKNESS. EXISTING ALLOTMENT DRAINAGE PIPE ARE TO BE BUILT INTO NEW KERB WITH A 100mm DIA HOLE. EXISTING KERBS ARE TO BE COMPLETELY REMOVED WHERE NEW KERBS ARE

RIGID CONCRETE PAVEMENT NOTES

GENERAL

- 1. CARRY OUT ALL CONCRETE WORK IN ACCORDANCE WITH AS3600-2018 AND THE SPECIFICATION, KEEP A COPY OF THESE DOCUMENTS ON SITE.
- 2. VERIFY ALL SETTING OUT DIMENSIONS WITH THE ARCHITECT AND OR THE SURVEYOR.
- 3. DO NOT OBTAIN DIMENSIONS BY SCALING THE DRAWINGS.
- IN CASE OF DOUBT ASK.

CONCRETE

PLACE CONCRETE OF THE FOLLOWING CHARACTERISTIC COMPRESSIVE STRENGTH F'C AS DEFINED IN AS3600-2018 OR RTA FORM 609. ADD WATER REDUCING ADMIXTURE EQUAL TO WRDA.

	AS3600 F'c MPa AT 28 DAYS	SPECIFIED SLUMP	NOMINAL AGG. SIZE	
ALL KERBS, PITS ETC.				
	25	80	20	
VEHICULAR PAVING	REFER TO CONCRETE PAVEMENT NOTES BELOW			

- 2. USE TYPE GP CEMENT IN ACCORDANCE WITH AS3972.
- 3. ALL CONCRETE SHALL BE SUBJECT TO PROJECT CONTROL SAMPLE AND TESTING TO AS3600-2018.
- 4. CONSOLIDATE BY VIBRATION. CURE SURFACES BY COVERING WITH PLASTIC AND KEEPING SURFACE MOIST FOR MIN 7 DAYS.

REINFORCEMENT:

- 1. FIX REINFORCEMENT AS SHOWN ON DRAWINGS. THE TYPE AND GRADE IS
- INDICATED BY A SYMBOL AS SHOWN BELOW.
- N. HOT ROLLED DEFORMED BAR. GRADE 500 R. PLAIN ROUND BAR. GRADE 250
- SL OR RL HARD DRAWN WIRE FABRIC SQUARE OR RECTANGULAR
- PROVIDE BAR SUPPORTS OR SPACERS TO GIVE THE FOLLOWING CONCRETE
- COVER TO ALL REINFORCEMENT UNLESS NOTED OTHERWISE. FOOTINGS: 75 BOTTOM, 65 TOP AND SIDES 40
- 40 WHEN EXPOSED TO WEATHER. SLABS:
- DRAINAGE STRUCTURES: 30 WHEN CAST IN FORMS BUT LATER EXPOSED TO WEATHER OR GROUND. 65 WHEN CAST DIRECTLY IN CONTACT WITH GROUND.
- 3. FOR ALL PITS LOCATED IN CONCRETE PROVIDE 2N16 TRIMMER BARS AROUND 1200mm LONG AROUND ALL CORNERS OF PITS UNLESS SHOWN OTHERWISE.

CONCRETE VEHICULAR PAVEMENT NOTES:

- CONCRETE MIX PARAMETERS:
- MAXIMUM AGGREGATE SIZE 20mm FLEXURAL STRENGTH AT 28 DAYS = 3.5 MPa (F'c=32MPa)
- FLEXURAL STRENGTH AT 90 DAYS = 3.85 MPa
- MAXIMUM WATER / CEMENT RATIO =0.45
- MAXIMUM SHRINKAGE LIMIT = 600 MICROSTRAINS (AS1012.13) AFTER 8 WEEKS OF DRYING
- MINIMUM CEMENT CONTENT = 300kg/m3
- CEMENT TO BE TYPE GP (NORMAL CEMENT) TO AS 3972 SLUMP = 50mm

EXCEEDS 35°C.

REQUIREMENTS.

- 2. SAWN JOINTS ARE TO BE CUT BETWEEN 2-4 HOURS AFTER CONCRETE POUR USING SOFF CUT SAW TO AVOID DAMAGING THE SURFACE DURING SAWCUT
- 3. FOR EXPANSION JOINTS, PRIOR TO THE PLACEMENT OF CONCRETE IN THE ADJACENT SLAB, SELF EXPANDING CORK FILLER SHALL BE ADHERED TO THE ALREADY CAST AND CLEANED CONCRETE FACE USING AN APPROVED WATERPROOF ADHESIVE. ADHESIVE SHALL BE LIBERALLY APPLIED TO THE FULL FACE OF THE CONCRETE SLAB TO BE COVERED BY THE FILLER. AND ON THE FULL FACE OF THE FILLER TO BE ADHERED. THE BUILDER SHALL PROVIDE CONSTANT SUPERVISION OF CONCRETE POURS EXECUTED BY SUB-CONTRACTORS TO ENSURE:
- REINFORCEMENT DISPLACED OFF CHAIRS ARE REPLACED PRIOR TO CONCRETE
- PLACEMENT. NO SITE WATER IS ADDED TO CONCRETE OR CONCRETE IN WAITING TRUCKS. (REQUIRED SLUMP FOR PLACEMENT SHALL BE ACHIEVED USING SUPER PLASTICISER)
- ALL CONCRETE IS FULLY COMPACTED USING A POKER VIBRATOR. NO POURS ARE EXECUTED WHEN THE AMBIENT TEMPERATURE EQUALS OR
- POURS ARE PROTECTED FROM ANY HOT DRYING WINDS.
- REFER TO COMPACTION NOTES FOR PREPARATION OF SUB-BASE AND SUB-GRADE. 5. CURING: THE FINISHED CONCRETE SHALL BE CURED FOR A MINIMUM OF
- SEVEN DAYS USING AT LEAST ONE OF THE FOLLOWING METHODS: PONDING OR CONTINUOUS SPRINKLING WITH WATER.
- THE USE OF AN ABSORBENT COVER KEPT CONSTANTLY WET. (WHEN THE AMBIENT) TEMPERATURE EXCEEDS 32°C CURING MAY ONLY BE ACHIEVED USING THE ABOVE
- THE USE OF AN IMPERMEABLE SHEET MEMBRANE OVER A MOISTENED SURFACE. (THE MEMBRANE SHALL BE FIXED AND LAPPED SO THAT NO AIR CIRCULATION CAN OCCUR AT THE CONCRETE SURFACE).
- THE USE OF A CURING COMPOUND COMPLYING WITH AS3799, APPLIED UNIFORMLY IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS, AND WHEN DRY THE COAT SHOULD BE CONTINUOUS, FLEXIBLE AND WITHOUT VISIBLE BREAKS OR PIN HOLES FOR SEVEN DAYS.

BROOM CONCRETE FINISH UNLESS SPECIFIED OTHERWISE TO ARCHITECTS

FOR DA ONLY

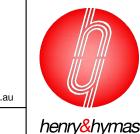
SURVEY NSW GOVERMENT LAND & HOUSING CORPORATION **INFORMATION** SURVEYED BY ISSUED FOR DA ONLY NW 06.06.2024 CUSTANCE ARCHITECTURE NW 23.02.2024 NORTON SURVEY PARTNERS 02 ISSUED FOR DA ONLY MP 01 ISSUED AS CONCEPT NW 15.12.2023 DATUM: A.H.D This drawing and design remains the property of Henry & Hymas and may not be copied in whole or in part without the prior written approval of Henry & Hymas. ORIGIN OF LEVELS: SSM135870 RL26.542 AMENDMENT DRAWN DESIGNED DATE REVISION AMENDMENT DRAWN DESIGNED DATE

828 Pacific Highway Gordon NSW 2072

Suite 2.01

+61 2 9417 8400 Facsimile +61 2 9417 8337 email@hhconsult.com.au www.henryandhymas.com.au

DRAWING TO BE PRINTED IN COLOUR



PROPOSED RESIDENTIAL DEVELOPMENT 17-27 HARDWICKE ST. RIVERWOOD, NSW **NOTES**

N.Wetzlar A.Francis

S.Chen

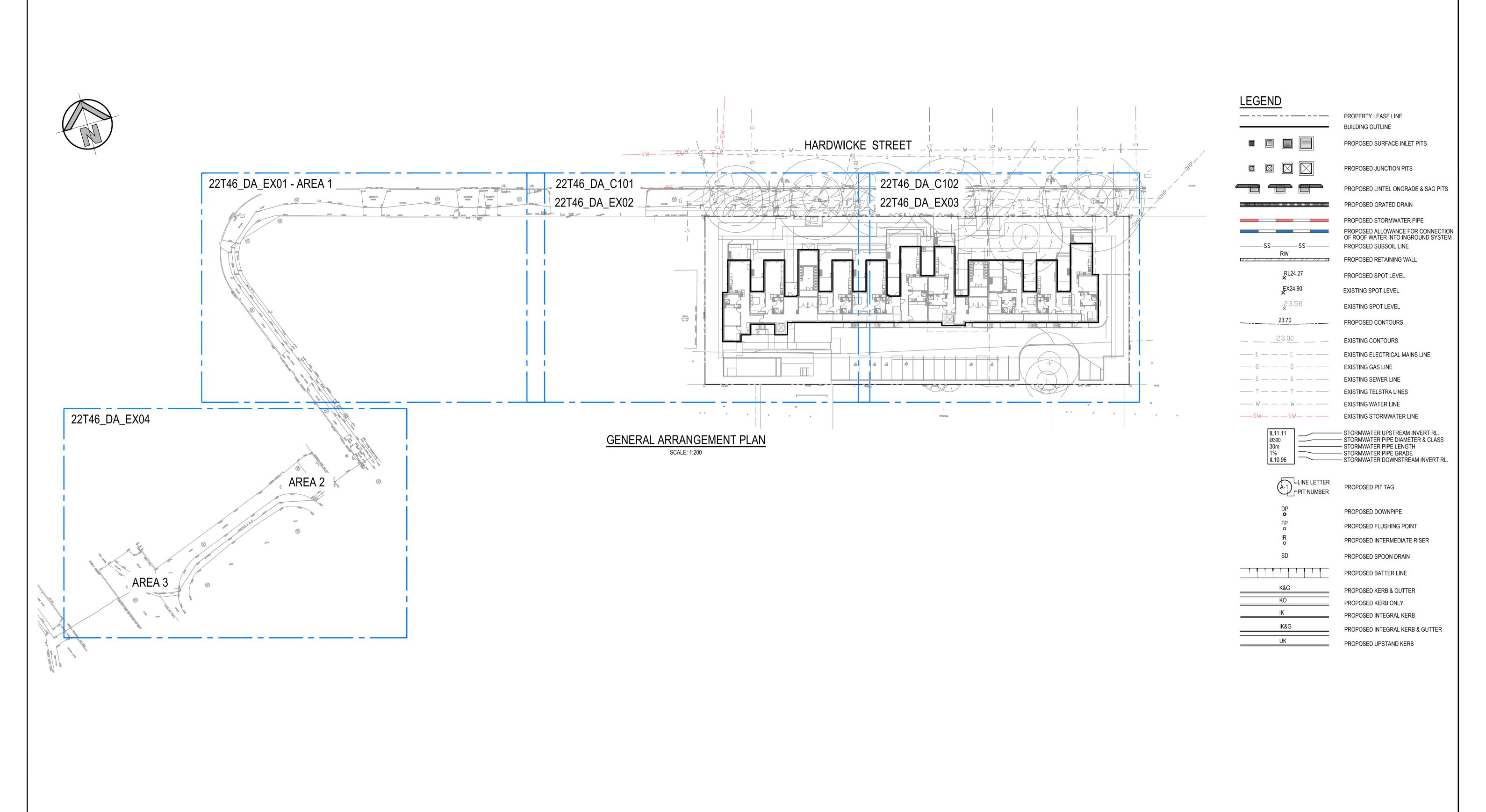
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N.Wetzlar

DECEMBER 2023

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N.T.S.



FOR DA ONLY SCALE 1:500

SURVEY NSW GOVERMENT LAND & HOUSING CORPORATION INFORMATION SURVEYED BY CUSTANCE ARCHITECTURE 03 ISSUED FOR DA ONLY NW 30.05.2024 NW 23.02.2024 NORTON SURVEY PARTNERS 02 ISSUED FOR DA ONLY NW 15.12.2023 01 ISSUED AS CONCEPT This drawing and design remains the property of Henry & Hymas and may not be copied in whole or in part without the prior written approval of Henry & Hymas. DATUM: A.H.D DRAWING TO BE PRINTED IN COLOUR ORIGIN OF LEVELS: SSM135870 RL26.542 AMENDMENT DRAWN DESIGNED DATE REVISION AMENDMENT DRAWN DESIGNED DATE

828 Pacific Highway	+61 2 9417 8400
Gordon NSW 2072	Facsimile +61 2 9417 8337
Silohal-Mark com au/®	Email email@hhconsult.c Web
Global-Mark.com.au&	www.henryandhyn

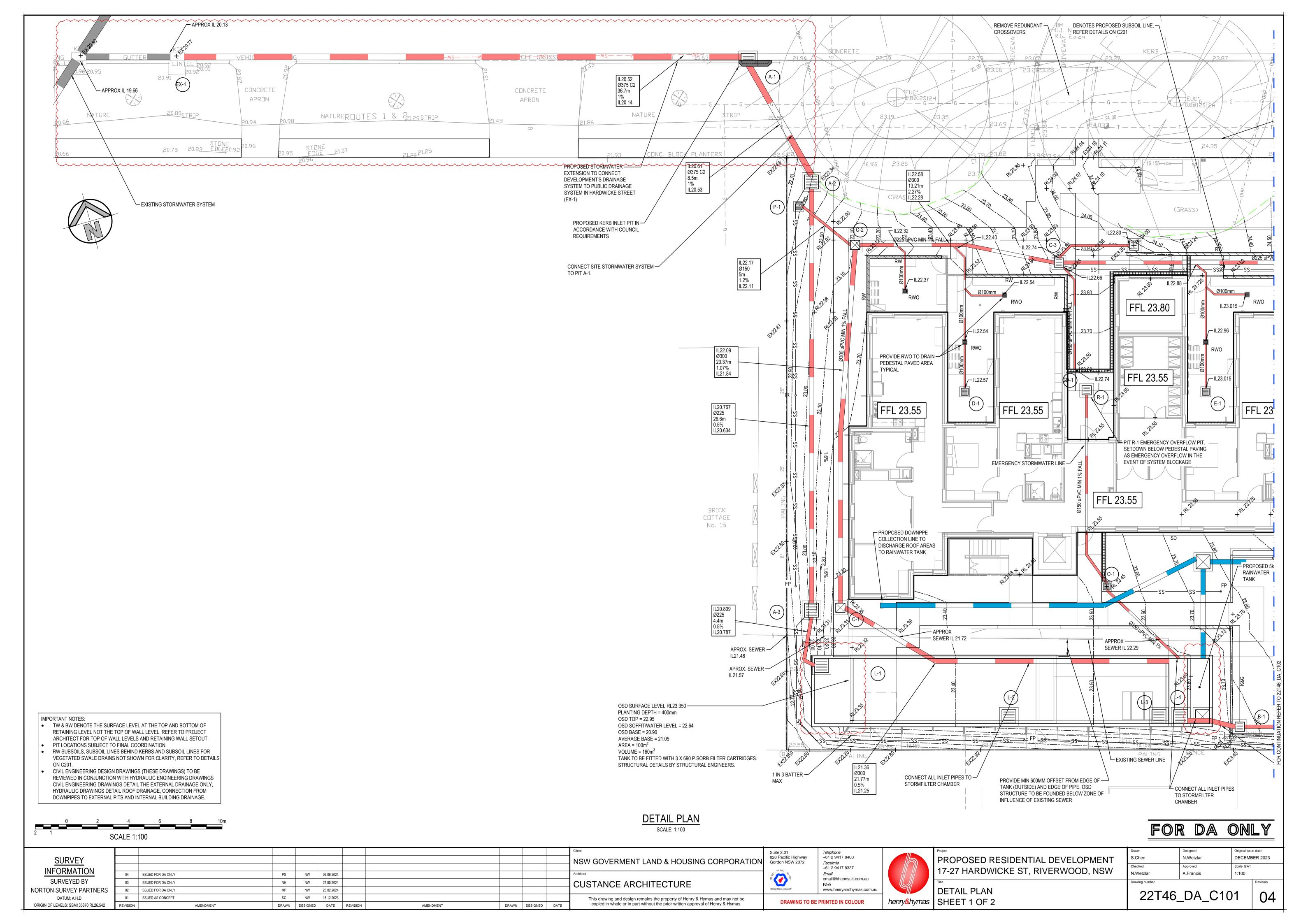
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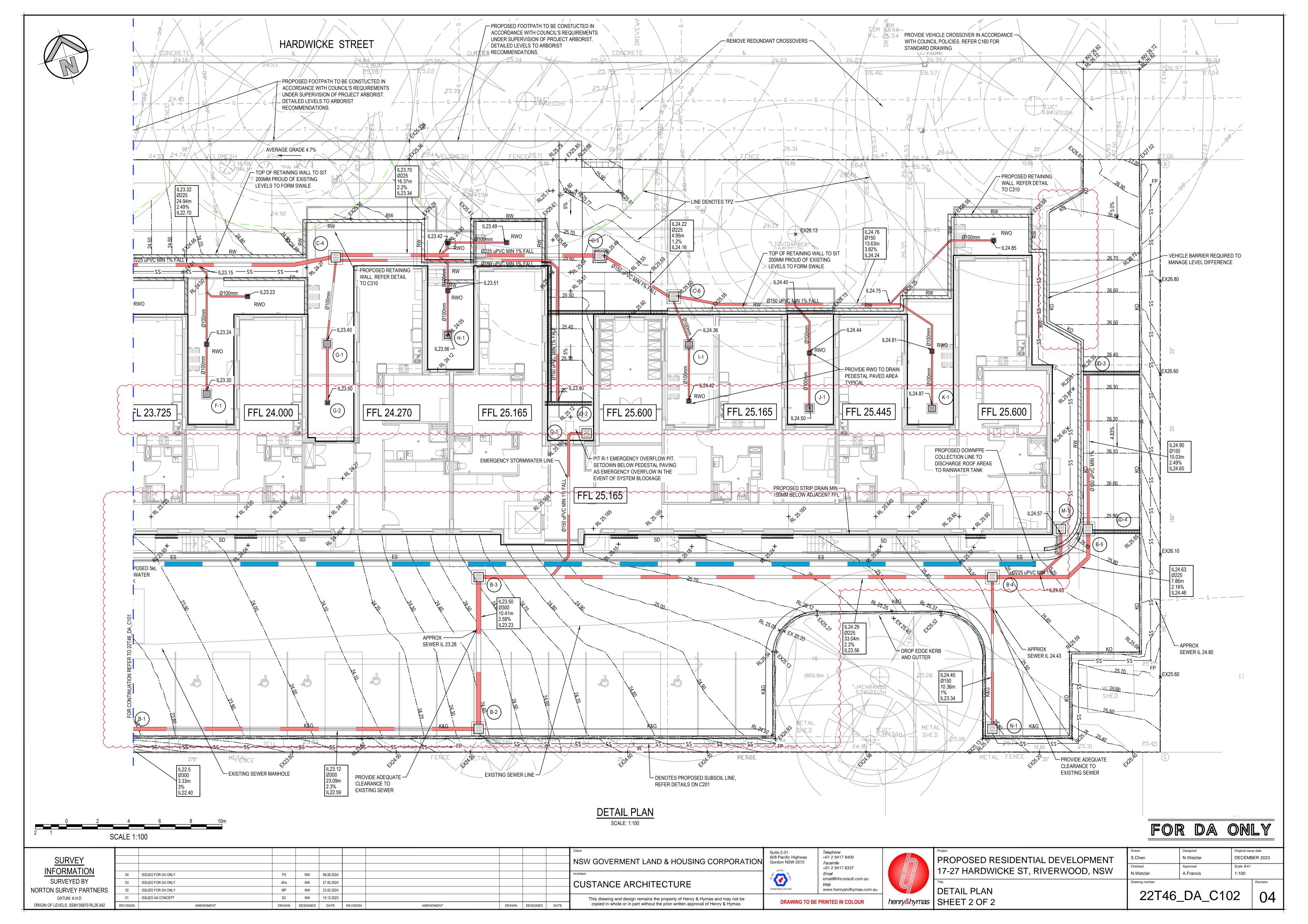
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GENERAL ARRANGEMENT PLAN	Drawing number 22T46_	_DA_C10	0

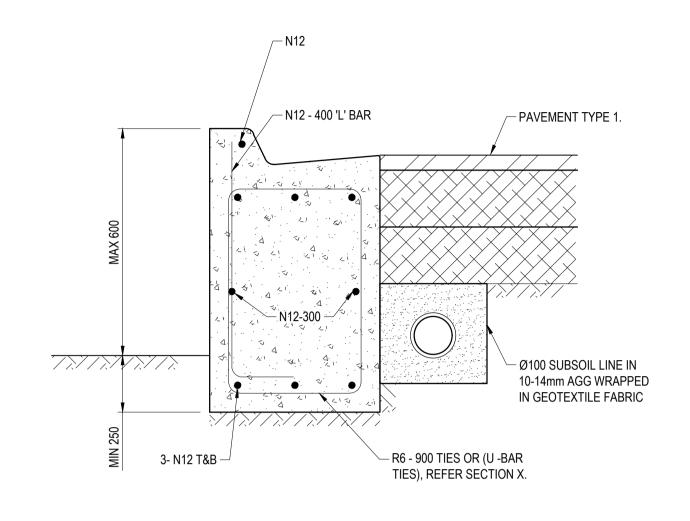
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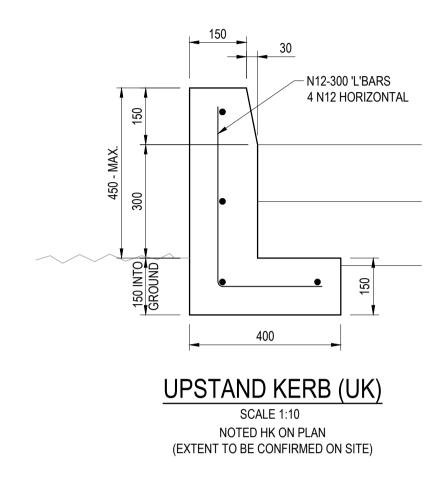
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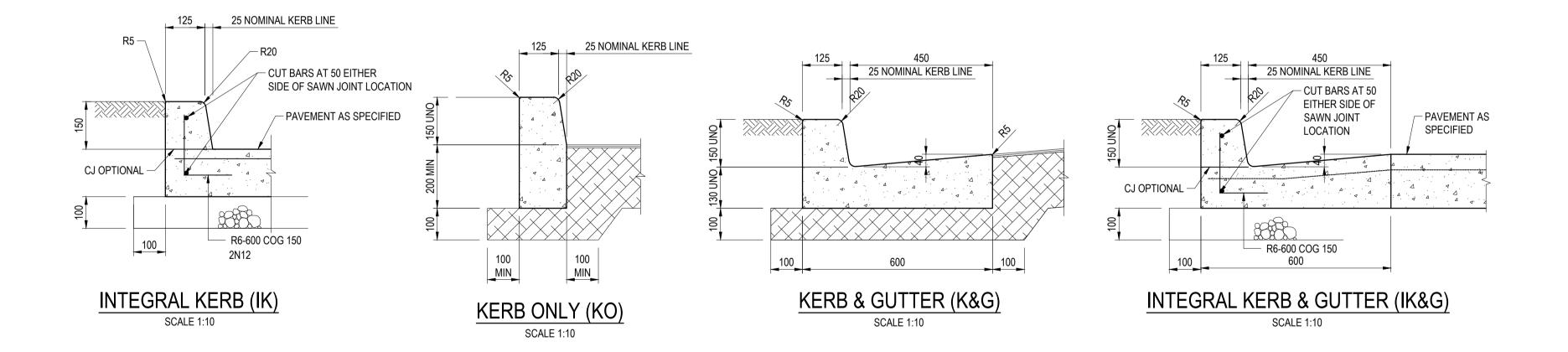








DROP EDGE KERB & GUTTER (DEK&G)

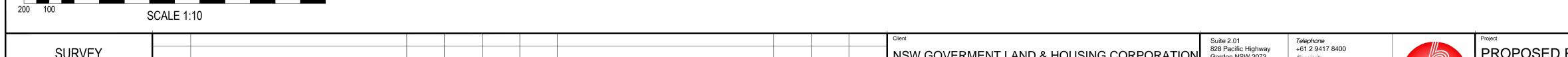


KERBING NOTES:

INCLUDES ALL KERBS, GUTTERS, DISH DRAINS, CROSSING AND EDGES.

- 1. ALL KERBS, GUTTERS, DISH DRAINS AND CROSSING TO BE CONSTRUCTED ON MINIMUM 75mm GRANULAR BASE COURSE COMPACTED TO MINIMUM 98% MODIFIED MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS 1289 5.2.1.
- 2. EXPANSION JOINTS (EJ) TO BE FORMED FROM 10mm COMPRESSIBLE CORK FILTER BOARD FOR THE FULL DEPTH OF THE SECTION AND CUT TO PROFILE. EXPANSION JOINTS TO BE LOCATED AT DRAINAGE PITS, ON TANGENT POINTS OF CURVES AND ELSEWHERE AT 12m CENTRES EXCEPT FOR INTEGRAL KERBS WHERE THE EXPANSION JOINTS ARE TO MATCH THE JOINT LOCATION IN
- 3. WEAKENED PLANE JOINTS TO BE MIN 3mm WIDE AND LOCATED AT 3m CENTRES EXCEPT FOR INTEGRAL KERBS WHERE WEAKENED PLANE JOINTS ARE TO MATCH THE JOINT LOCATIONS IN SLABS.
- 4. BROAMED FINISHED TO ALL RAMPED AND VEHICULAR CROSSINGS, ALL OTHER KERBING OR DISH DRAINS TO BE STEEL FLOAT FINISHED.
- 5. IN THE REPLACEMENT OF KERBS EXISTING ROAD PAVEMENT IS TO BE SAWCUT 900mm FROM LIP OF GUTTER. UPON COMPLETION OF NEW KERBS, NEW BASE COURSE AND SURFACE IS TO BE LAID 900mm WIDE TO MATCH EXISTING MATERIALS AND THICKNESS. EXISTING ALLOTMENT DRAINAGE PIPE ARE TO BE BUILT INTO NEW KERB WITH A 100mm DIA HOLE.

EXISTING KERBS ARE TO BE COMPLETELY REMOVED WHERE NEW KERBS ARE



SURVEY **NSW GOVERMENT LAND & HOUSING CORPORATION** INFORMATION SURVEYED BY CUSTANCE ARCHITECTURE NORTON SURVEY PARTNERS 01 ISSUED FOR DA ONLY MP NW 23.02.2024 DATUM: A.H.D This drawing and design remains the property of Henry & Hymas and may not be copied in whole or in part without the prior written approval of Henry & Hymas. ORIGIN OF LEVELS: SSM135870 RL26.542 AMENDMENT DRAWN DESIGNED DATE REVISION AMENDMENT DRAWN DESIGNED DATE



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PROPOSED RESIDENTIAL DEVELOPMENT 17-27 HARDWICKE ST, RIVERWOOD, NSW

TYPICAL KERB AND CONCRETE DETAILS

FOR DA ONLY S.Chen N.Wetzlar DECEMBER 2023

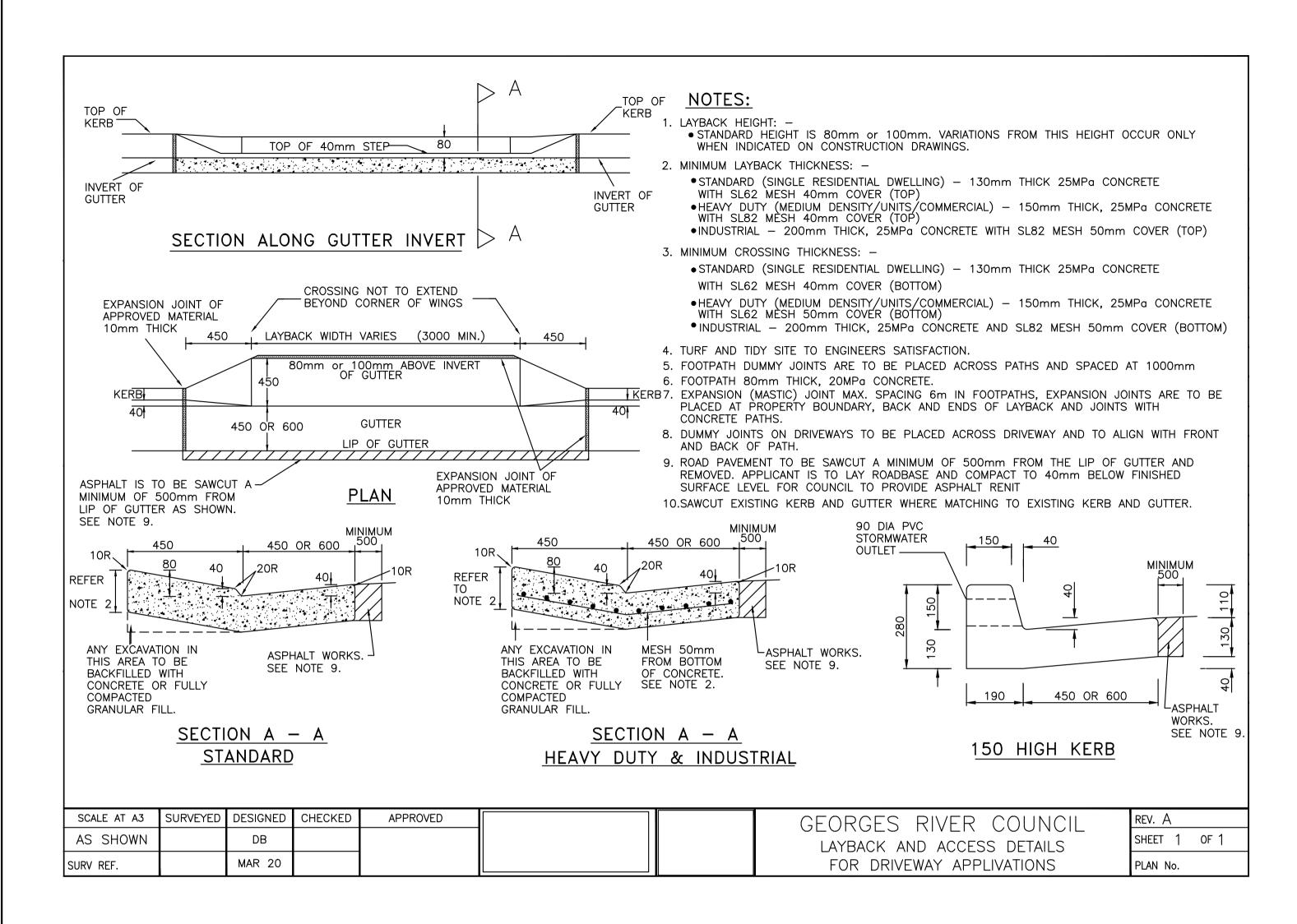
A.Francis

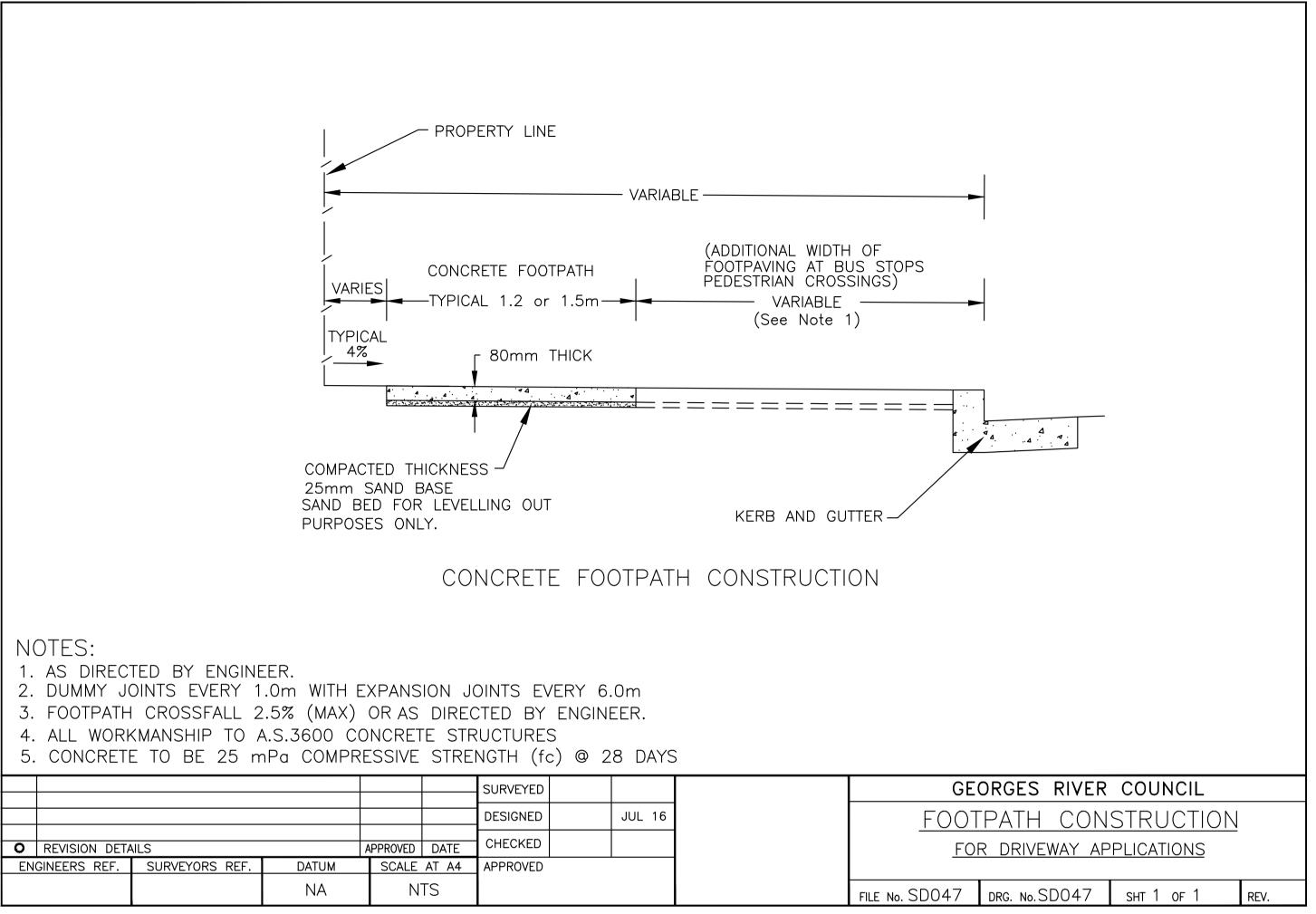
N.Wetzlar

22T46_DA_C110

Scale @A1

AS NOTED

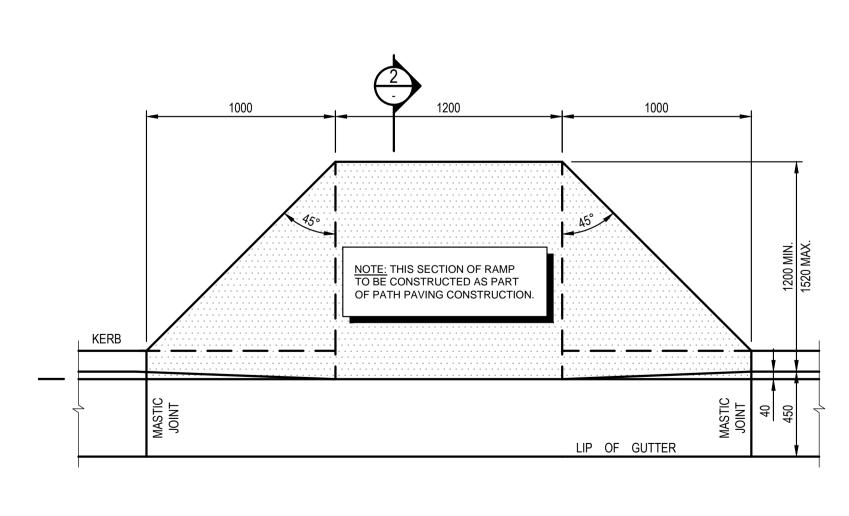


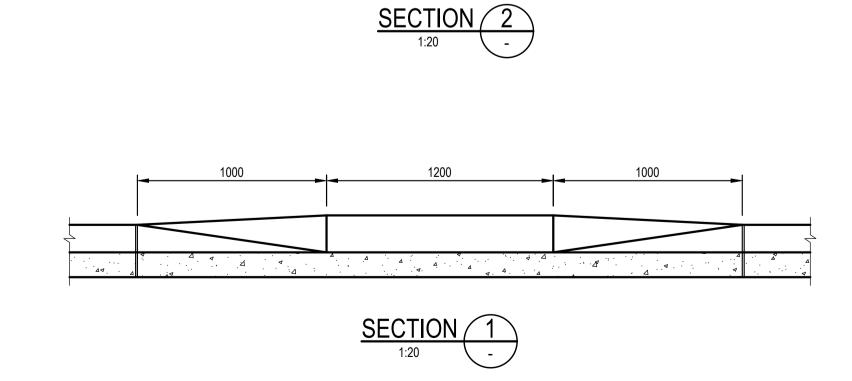


CURTAILED PRAM RAMP

- MIN. 175mm COMPACTED

LAYER OF DGB20





150

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THIS SECTION OF RAMP TO BE CONSTRUCTED AS PART OF PATH PAVING CONSTRUCTION

RAMP SHALL HAVE A NON-SLIP FINISH. THE —
DEPTH OF PAVING SHALL CONFORM TO

ADJACENT FOOTPATH PAVING.

25mm OF APPROVED —

GRANULAR MATERIAL

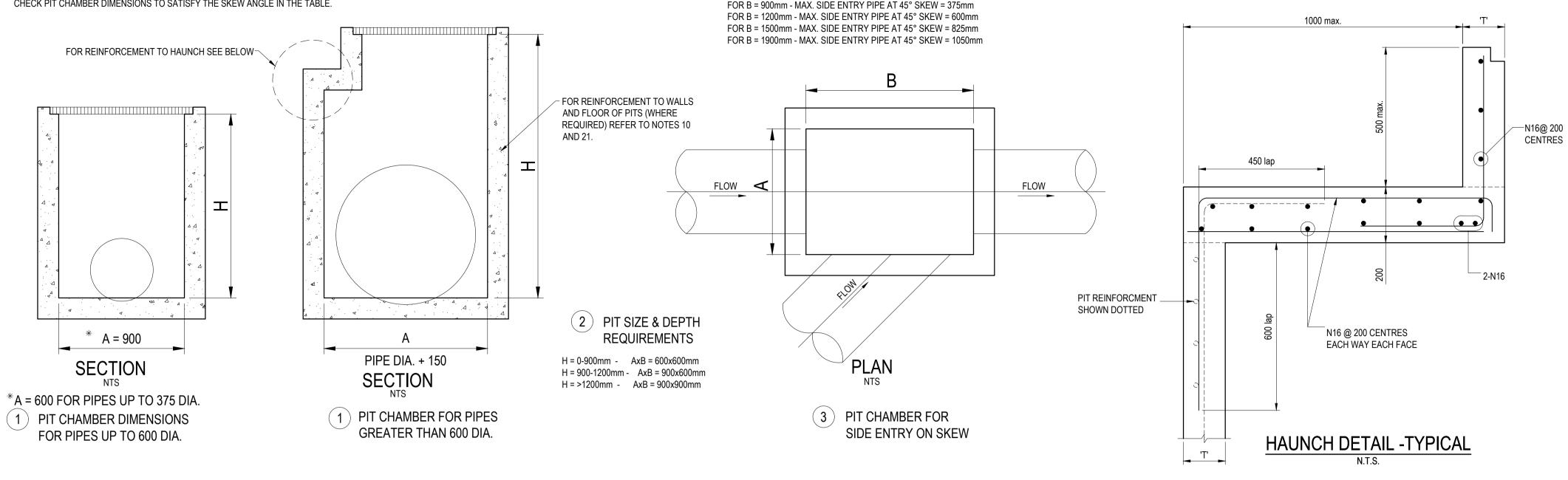
PRAM RAMP DETAIL SCALE 1:20

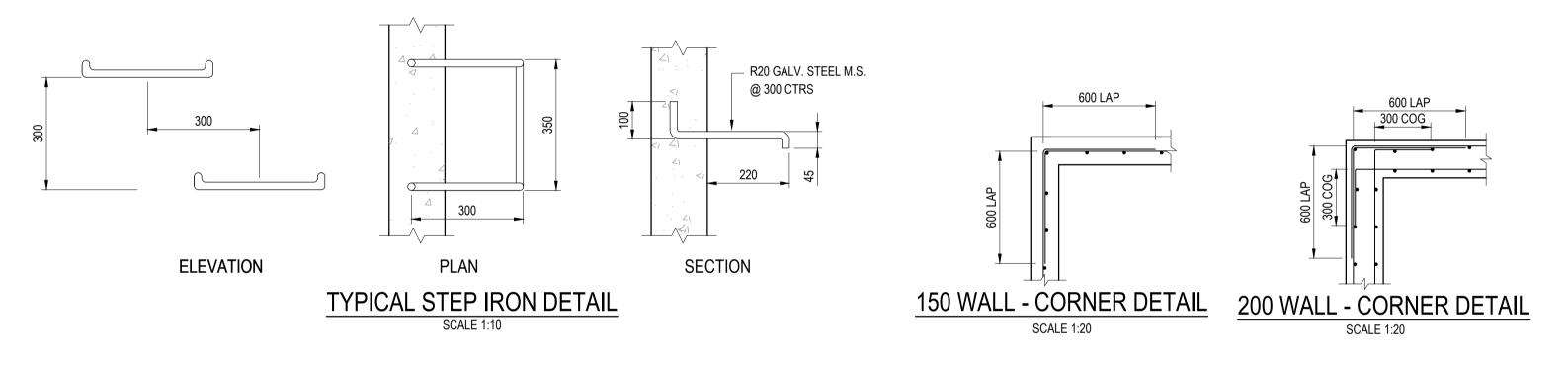
SCALE 1:20 828 Pacific Highway SURVEY NSW GOVERMENT LAND & HOUSING CORPORATION Gordon NSW 2072 INFORMATION SURVEYED BY ISSUED FOR DA ONLY NW 06.06.2024 CUSTANCE ARCHITECTURE NW 23.02.2024 NORTON SURVEY PARTNERS ISSUED FOR DA ONLY NW 15.12.2023 01 ISSUED AS CONCEPT DATUM: A.H.D This drawing and design remains the property of Henry & Hymas and may not be copied in whole or in part without the prior written approval of Henry & Hymas. ORIGIN OF LEVELS: SSM135870 RL26.542 DRAWN DESIGNED DATE REVISION AMENDMENT AMENDMENT DRAWN DESIGNED DATE

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Project	Drawn	Designed	Original issu	ie date
PROPOSED RESIDENTIAL DEVELOPMENT	S.Chen	N.Wetzlar	DECEME	3ER 2023
47 07 LIADDIA/ICKE CT DIVEDIA/OOD NICK	Checked	Approved	Scale @A1	
17-27 HARDWICKE ST, RIVERWOOD, NSW	N.Wetzlar	A.Francis	AS NOTE	ΕD
Title	Drawing number		•	Revision
STANDARD DRAWINGS	22T46	DA C16	· O	00
CROSSOVER, FOOTPATH	ZZ 140 <u>.</u>	_DA_C16)U	03
CRUSSUVER, FUUTFATH				

TYPICAL PIT CHAMBER SIZES IT IS THE CONTRACTORS RESPONSIBILITY TO SELECT PIT CHAMBER SIZE WITH REGARDS TO PIPE SIZE, DEPTH TO INVERT AND SKEW ANGLE. REFER SKETCHES BELOW. 1 SELECT PIT CHAMBER USING THE STEPS BELOW: SELECT PIT CHAMBER SIZE DEPENDING ON THE PIPE DIAMETERS. 3 CHECK PIT CHAMBER SIZE TO SATISFY DEPTH TO INVERT REQUIREMENTS. FOR B = 600mm - MAX. SIDE ENTRY PIPE AT 45° SKEW = 225mm CHECK PIT CHAMBER DIMENSIONS TO SATISFY THE SKEW ANGLE IN THE TABLE. FOR B = 900mm - MAX. SIDE ENTRY PIPE AT 45° SKEW = 375mm FOR B = 1200mm - MAX. SIDE ENTRY PIPE AT 45° SKEW = 600mm FOR B = 1500mm - MAX. SIDE ENTRY PIPE AT 45° SKEW = 825mm





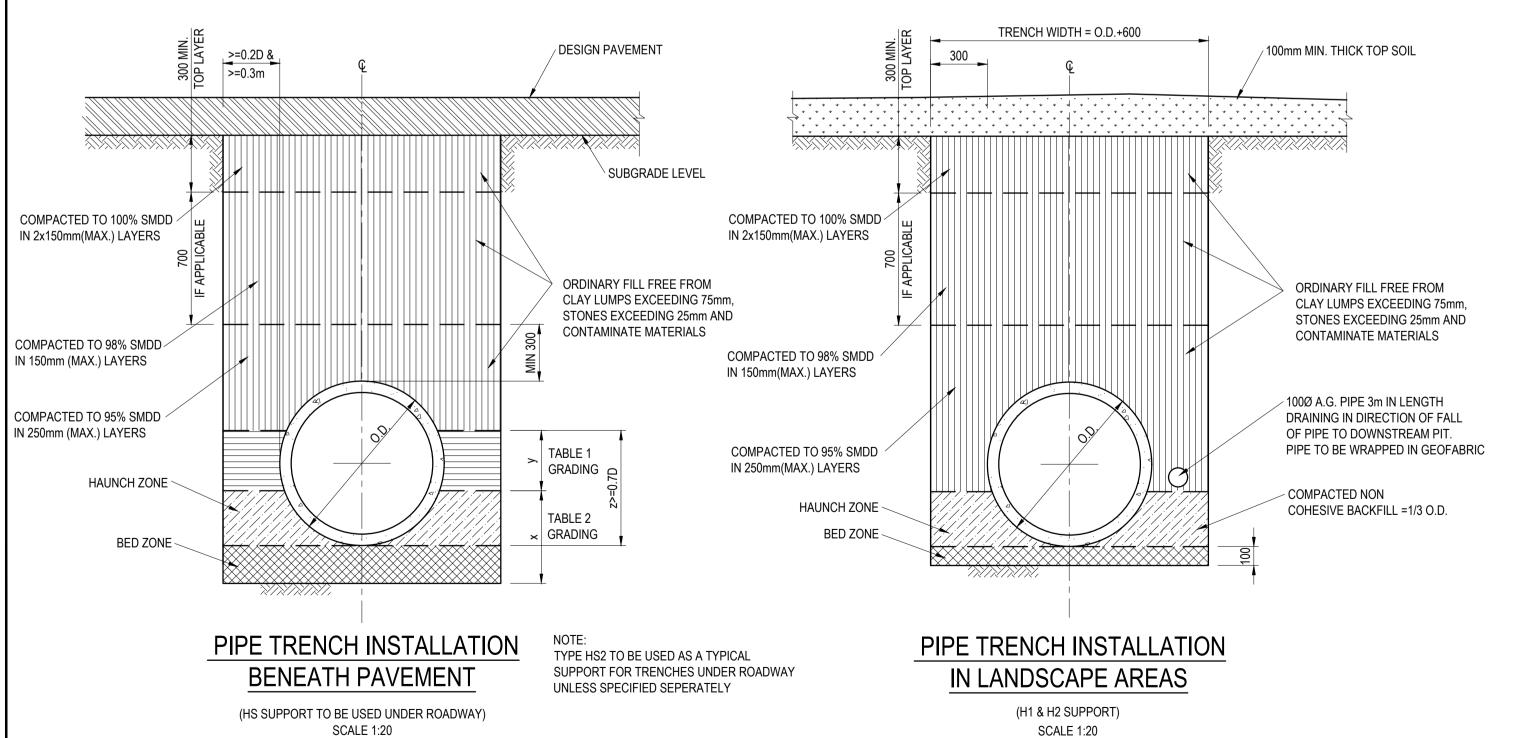


TABLE 4				
TABLE 1				
SIEVE SIZE (MM)	WEIGHT PASISNG (%)			
75.0	100			
9.5	100 TO 50			
2.36	100 TO 30			
0.60	50 TO 15			
0.075	25 TO 0			

TABLE 2				
SIEVE SIZE (MM)	WEIGHT PASISNG (%)			
19.0	100			
2.36	100 TO 50			
0.60	90 TO 20			
0.30	60 TO 10			
0.15	25 TO 0			
0.075	10 TO 0			

	TABLE 3					
SUPPORT TYPE	BED ZONE X	HAUNCH ZONE Y	BED AND HAUNCH ZONES COMPACTION	MAX BEDDING FACTOR		
HS1		0.1D	50	2.0		
HS2	100 IF D<=1500, OR 150 IF D>=1500	0.3D	60	2.5		
HS3		0.3D	70	4.0		

PIT LID AND HEADWALL SCHEDULE

PIT/STRUCTURE NUMBER	DESCRIPTION
A-2 A-3	PROPOSED 900x900 SURFACE INLET PIT WITH HINGED GRATED LID LIGHT DUTY CLASS "B" IN ACCORDANCE WITH CAMDEN COUNCIL'S REQUIREMENTS.
$ \begin{array}{c c} \hline D-1 & \hline E-1 & \hline G-1 & \hline \end{array} $ $ \begin{array}{c c} \hline G-1 & \hline \end{array} $ $ \begin{array}{c c} \hline H-1 & \hline \end{array} $ $ \begin{array}{c c} \hline \end{array} $	PROPOSED 450x450 SURFACE INLET PIT WITH HINGED GRATED LID LIGHT DUTY CLASS "B" IN ACCORDANCE WITH CAMDEN COUNCIL'S REQUIREMENTS.
$ \begin{array}{c c} \hline & C-3 \\ \hline & C-4 \\ \hline & C-5 \\ \hline & C-6 \\ \hline & Q-1 \\ \hline & R-1 \\ \hline \end{array} $	PROPOSED 600x600 SURFACE INLET PIT WITH HINGED GRATED LID LIGHT DUTY CLASS "B" IN ACCORDANCE WITH CAMDEN COUNCIL'S REQUIREMENTS.
B-1 B-2 L-1 L-2 L-3 L-4	PROPOSED 900x900 SURFACE INLET PIT WITH HINGED GRATED LID MEDIUM DUTY CLASS "C" IN ACCORDANCE WITH CAMDEN COUNCIL'S REQUIREMENTS.
B-3 (B-5) (N-1) (N-4)	PROPOSED 600x600 SURFACE INLET PIT WITH HINGED GRATED LID MEDIUM DUTY CLASS "C" IN ACCORDANCE WITH CAMDEN COUNCIL'S REQUIREMENTS.
C-2 C-1	PROPOSED JUNCTION PIT WITH 900x900 MEDIUM DUTY SEALED LID CLASS "C" IN ACCORDANCE WITH CAMDEN COUNCIL'S REQUIREMENTS.
A-1	PROPOSED 1.2m KERB INLET PIT WITH 900x450 INLET GRATE HEAVY DUTY CLASS 'D' IN ACCORDANCE WITH CAMDEN COUNCIL'S REQUIREMENTS.
GD-3) GD-4)	200mm WIDE HEAVY DUTY GRATED DRAIN AND FRAME CLASS "D" IN ACCORDANCE WITH CAMDEN COUNCIL'S REQUIREMENTS
GD-1) GD-2)	150mm WIDE LIGHT DUTY GRATED DRAIN TO ARCHITECTS SPECIFICATIONS
EX-1	EXISTING PIT TO BE MODIFIED, REFER NOTES ON PLAN

NOTES:

- ALL GRATED INLET PITS TO CONTAIN OCEAN PROTECT 200 MICRON PIT BASKETS OR APPROVED EQUIVALENT UNLESS NOTED ON PLAN. REFER DRAWING C201 FOR DETAILS.
- REFER C201 FOR DRAINAGE AND SUBSOIL NOTES.
- ALL PITS IN FOOTPATH, OR ADJACENT TO FOOTPATHS, TO BE FITTED WITH SLIP RESISTANT HEELGUARD LID.
- PIT SCHEDULE ABOVE DOES NOT INCLUDE PIT LIDS FOR WATER QUALITY DEVICES OR BELOW GROUND OSD TANKS
- ALL RAINWATER OUTLETS (RWO) TO BE MIN 200MM IN DIAMETER. ALL RAINWATER OUTLETS TO BE HEELPROOF FINISH AND TO ARCHITECTS SPECIFICATIONS UNLESS NOTED ON PLAN.

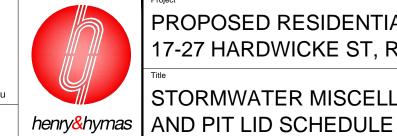
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SCALE 1:10 SCALE 1:20 SURVEY **NSW GOVERMENT LAND & HOUSING CORPORATION** INFORMATION SURVEYED BY CUSTANCE ARCHITECTURE NORTON SURVEY PARTNERS ISSUED FOR DA ONLY 23.02.2024 NW 15.12.2023 ISSUED AS CONCEPT DATUM: A.H.D This drawing and design remains the property of Henry & Hymas and may not be copied in whole or in part without the prior written approval of Henry & Hymas. ORIGIN OF LEVELS: SSM135870 RL26.542 DRAWN DESIGNED DATE REVISION AMENDMENT AMENDMENT DRAWN DESIGNED DATE

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PROPOSED RESIDENTIAL DEVELOPMENT 17-27 HARDWICKE ST, RIVERWOOD, NSW STORMWATER MISCELLANEOUS DETAILS

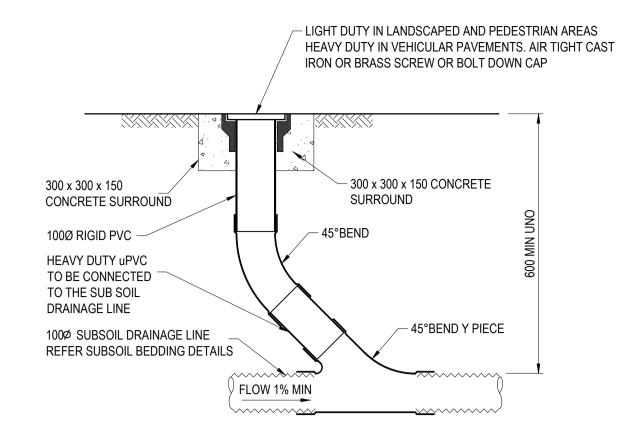
N.Wetzlar N.Wetzlar A.Francis

22T46_DA_C200

DECEMBER 2023

Scale @A1

1:200



INTERMEDIATE RISER (IR)

SCALE 1:10 NOTE: SLOTTED RIGID PVC PIPE AND FITTINGS MAY BE USED

LIGHT DUTY IN LANDSCAPED AND PEDESTRIAN AREAS HEAVY DUTY IN VEHICULAR PAVEMENTS. AIR TIGHT CAST IRON OR BRASS SCREW OR BOLT DOWN CAP. - 300x300x150 CONCRETE SURROUND 100 DIA RIGID PVC -HEAVY DUTY uPVC TO BE CONNECTED

TO THE SUB SOIL DRAINAGE LINE

FLUSHING POINT (FP)

SCALE 1:10 NOTE: SLOTTED RIGID PVC PIPE AND FITTINGS MAY BE USED

SUBSOIL DRAINAGE NOTES

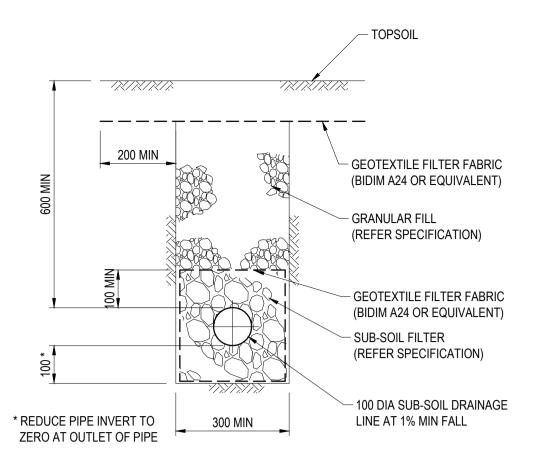
- 1. GENERALY PROVIDE SUBSOIL DRAINS TO INTERCEPT GROUNDWATER SEEPAGE AND PREVENT WATER BUILD-UP BEHIND WALLS AND UNDER FLOORS AND PAVEMENTS. CONNECT SUBSOIL TO SURFACE DRAINS OR TO THE STORMWATER DRAINAGE SYSTEM AS APPLICABLE.
- 2. PIPE DEPTH: PROVIDE THE FOLLOWING MINIMUM CLEAR DEPTH, MEASURED TO THE CROWN OF THE PIPE, WHERE THE PIPE PASSES BELOW THE FOLLOWING ELEMENTS:
- 100mm BELOW FORMATION LEVEL OF THE PAVEMENT, KERB OR
- 100mm BELOW THE AVERAGE GRADIENT OF THE BOTTOM OF FOOTINGS. JOINTING:
- AT JUNCTIONS OF SUBSOIL PIPES PROVIDE TEES, COUPLINGS OR ADAPTORS TO AS2439.1.
- 4. TRENCH WIDTH MINIMUM 300mm

CHANNEL

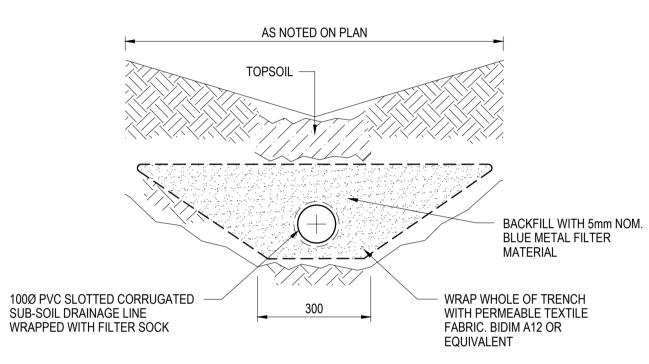
PIPE UNDERLAY GENERAL: GRADE THE TRENCH FLOOR EVENLY TO THE GRADIENT OF THE PIPELINE. IF THE TRENCH FLOOR IS ROCK, CORRECT ANY IRREGULARITIES WITH COMPACTED BEDDING MATERIAL. BED PIPING ON A CONTINUOUS UNDERLAY OF BEDDING MATERIAL, AT LEAST 75mm THICK AFTER COMPACTION. LAY THE PIPE WITH ONE LINE OF PERFORATIONS AT THE BOTTOM.

CHASES: IF NECESSARY TO PREVENT PROJECTIONS SUCH AS SOCKETS AND FLANGES FROM BEARING ON THE TRENCH BOTTOM OR UNDERLAY.

- PIPE SURROUNDS: GENERAL: PLACE THE MATERIAL IN THE PIPE SURROUND IN LAYERS SMALLER THAN OR EQUAL TO 200mm LOOSE THICKNESS, AND COMPACT WITHOUT DAMAGING OR DISPLACING PIPING. DEPTH OF OVERLAY: TO THE UNDERSIDE OF THE BASE OF OVERLYING STRUCTURES SUCH AS PAVEMENTS, SLABS AND CHANNELS TO WITHIN 150mm OF THE FINISHED SURFACE OF UNPAVED OR LANDSCAPED AREAS.
- FILTER SOCKS: PROVIDE POLYESTER PERMEABLE SOCKS CAPABLE OF RETAINING PARTICLES OF 0.25mm SIZES. SECURELY FIT OR JOIN THE SOCK AT EACH

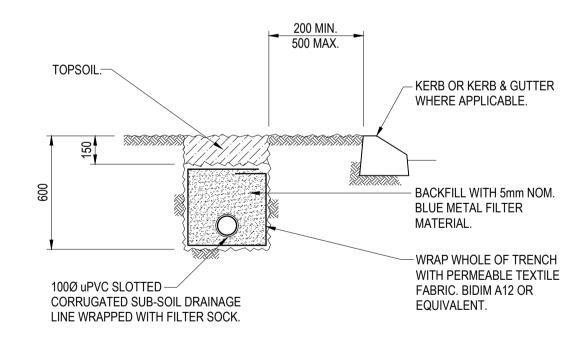


SUB-SOIL IN LANDSCAPED AREAS SCALE 1:10



SUBSOIL DRAIN DETAIL IN SWALES

SWALE SUBSOIL NOT SHOWN ON PLAN FOR CLARITY.



SUB-SOIL DRAIN DETAIL

DRAINAGE NOTES:

1. ALL STORMWATER WORK TO COMPLY WITH AS 3500 PART 3.

2. CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE MINIMUM COVER OF 600mm ON

3. PROTECTION OF PIPES DUE TO LOADS EXCEEDING W7 WHEEL LOAD SHALL BE THE CONTRACTOR'S RESPONSIBILITY.

4. BEDDING TYPE SHALL BE TYPE H2 FOR RCP. WHERE NECESSARY THE OVERLAY ZONE SHALL BE REDUCED TO ACCOMMODATE PAVEMENT REQUIREMENTS. REFER TO THIS DRAWING FOR

5. MINIMUM COVER OVER EXISTING PIPES FOR PROTECTION DURING CONSTRUCTION SHALL BE

6. NO CONSTRUCTION LOADS SHALL BE APPLIED TO PLASTIC PIPES.

7. FINISHED SURFACE LEVELS SHOWN ON LAYOUT PLAN DRGS TAKE PRECEDENCE OVER DESIGN DRAINAGE SURFACE LEVELS.

8. ALL PIPES UP TO AND INCLUDING 300 DIA. SHALL BE SOLVENT OR RUBBER RING JOINTED PVC CLASS SH PIPE TO AS1260. ALL OTHER PIPES TO BE RCP USING CLASS 2 RUBBER RING JOINTED PIPE. HARDIES FRC PIPE MAY BE USED IN LIEU OF RCP IF DESIRED IN GROUND. ALL AERIAL PIPES TO BE PVC CLASS SH.

9. ALL PITS IN NON TRAFFICABLE AREAS TO BE PREFABRICATED POLYESTER CONCRETE "POLYCRETE" WITH "LIGHT DUTY" CLASS B GALV. MILD STEEL GRATING AND FRAME. ALL PITS IN TRAFFICABLE AREAS (CLASS "D" LOADING MAX) TO HAVE 150mm THICK CONCRETE WALLS AND BASE CAST IN-SITU fc=32 MPa, REINFORCED WITH N12-200 BOTH LOADING WAYS CENTRALLY PLACE .U.N.O. ON SEPARATE DESIGN DRAWINGS IN THIS SET. GALV.MILD STEEL GRATING AND FRAME TO SUIT DESIGN LOADING. PRECAST PITS, RECTANGULAR OR CIRCULAR IN SHAPE, MAY BE USED IN LIEU AND SHALL COMPLY WITH RELEVANT AUSTRALIAN STANDARDS.

10. ALL PITS, GRATINGS AND FRAMES SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATION AND TO BE IN ACCORDANCE WITH AS3500.3 AND AS3996.

11. PIT CHAMBER DIMENSIONS ARE TO BE SELECTED TO SATISFY THE FOLLOWING: - PIPE SIZE

- DEPTH TO INVERT

- SKEW ANGLE

REFER TYPICAL PIT CHAMBER DETAILS BELOW IF PIT LID SIZE IS SMALLER THAN THE PIT CHAMBER SIZE THEN THE PIT LID IS TO BE CONSTRUCTED ON THE CORNER OF THE PIT CHAMBER WITH THE STEP IRONS DIRECTLY BELOW. ALTERNATIVELY THE PIT LID TO BE USED, IS TO BE THE SAME SIZE AS THE PIT CHAMBER.

12. FOR PIPE SIZES GREATER THAN Ø300mm, PIT FLOOR IS TO BE BENCHED TO FACILITATE FLOW.

13. GALVANISED STEP IRONS SHALL BE PROVIDED AT 300 CTS FOR PITS HAVING A DEPTH EXCEEDING 1200mm. SUBSOIL DRAINAGE PIPE SHALL BE PROVIDED IN PIPE TRENCHES ADJACENT TO INLET PIPES. (MINIMUM LENGTH 3m).

14. ALL SUBSOIL PIPES SHALL BE 100mm SLOTTED PVC IN A FILTER SOCK, UNO, WITH 3m INSTALLED UPSTREAM OF ALL PITS.

15. ALL PIPEWORK SHALL HAVE MINIMUM DIAMETER 100.

16. MINIMUM GRADE FOR ROOFWATER DRAINAGE LINES SHALL BE 1%.

17. ALL PIPE JUNCTIONS AND TAPER UP TO AND INCLUDING 300 DIA. SHALL BE VIA PURPOSE

18. ALL ROOF DRAINAGE TO BE INSTALLED IN ACCORDANCE WITH AS3500, PART 3. TESTING TO BE UNDERTAKEN AND REPORTS PROVIDED TO THE SUPERINTENDENT.

19. LOCATION OF THE DIRECT DOWN PIPE CONNECTIONS MAY VARY ON SITE TO SUIT SITE CONDITIONS, WHERE CONNECTION SHOWN ON LONG SECTIONS CHAINAGES ARE INDICATIVE

20. PITS IN EXCESS OF 1.5 m DEEP TO HAVE WALL AND FLOOR THICKNESS INCREASED TO 200mm. REINFORCED WITH N12@200 CTS CENTRALLY PLACED BOTH WAYS THROUGHOUT U.N.O.ON SEPARATE DESIGN DRAWINGS IN THIS SET. IF DEPTH EXCEEDS 5m CONTACT ENGINEER.

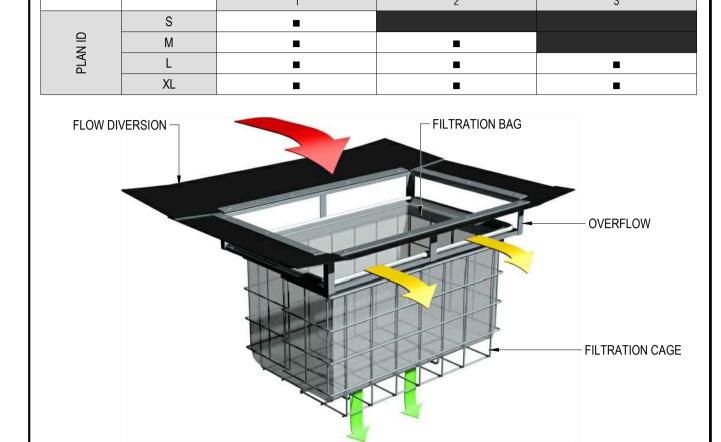
21. SUBSOIL DRAINAGE LINES FOR LANDSCAPE AREA NOT SHOWN ON THESE DRAWINGS. REFER TO LANDSCAPING PLANS FOR DETAILS.

22. ALL STORMWATER PITS TO HAVE Ø100 uPVC SLOTTED SUBSOIL PIPES CONNECTED TO THEM. THESE SUBSOILS TO EXTEND 3m UPSTREAM OF THE PIT AT A MINIMUM GRADE.

PLAN ID MAXIMUM PIT PLAN DIMENSIONS S 450mm x 450mm 600mm x 600mm 900mm x 900mm 1200mm x 1200mm

ı	DEPTH ID	BAG DEPTH	OVERALL DEPTH
	1	170	270
I	2	300	450
	3	600	700
1			

DEPTH ID

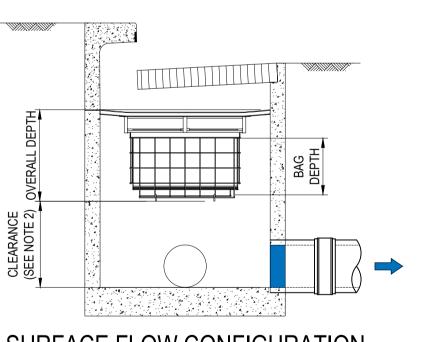


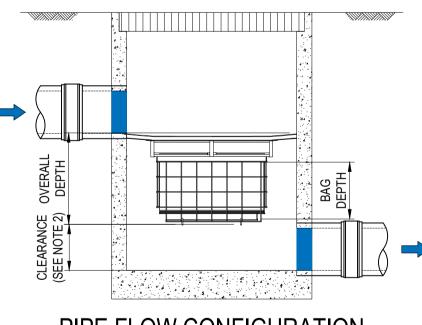
GENERAL NOTES

- THE MINIMUM CLEARANCE DEPENDS ON THE CONFIGURATION (SEE NOTE 2) AND THE LOCAL COUNCIL REQUIREMENTS.
- CLEARANCE FOR ANY PIT WITHOUT AN INLET PIPE (ONLY USED FOR SURFACE FLOW) CAN BE AS LOW AS 50mm. FOR OTHER PITS, THE RECOMMENDED CLEARANCE SHOULD BE GREATER OR EQUAL TO THE PIPE OBVERT SO AS NOT TO INHIBIT HYDRAULIC CAPACITY.
- OCEAN PROTECT PROVIDES TWO FILTRATION BAG TYPES:- 200 MICRON BAGS FOR HIGHER WATER QUALITY FILTERING AND A COARSE BAG FOR TARGETING GROSS POLLUTANTS.
- 4. DRAWINGS NOT TO SCALE.



OCEAN PROTECT OCEANGUARD TYPCIAL ARRANGEMENTS SPECIFICATION DRAWING





SURFACE FLOW CONFIGURATION SCALE: 1:20

PIPE FLOW CONFIGURATION SCALE: 1:20

FOR DA ONLY

SCALE 1:20 SCALE 1:10 SURVEY NSW GOVERMENT LAND & HOUSING CORPORATION **INFORMATION** SURVEYED BY CUSTANCE ARCHITECTURE NORTON SURVEY PARTNERS ISSUED FOR DA ONL' NW 23.02.2024 ISSUED AS CONCEPT NW 15.12.2023 DATUM: A.H.D This drawing and design remains the property of Henry & Hymas and may not be copied in whole or in part without the prior written approval of Henry & Hymas. ORIGIN OF LEVELS: SSM135870 RL26.542 DRAWN DESIGNED DATE REVISION AMENDMENT AMENDMENT DRAWN DESIGNED DATE

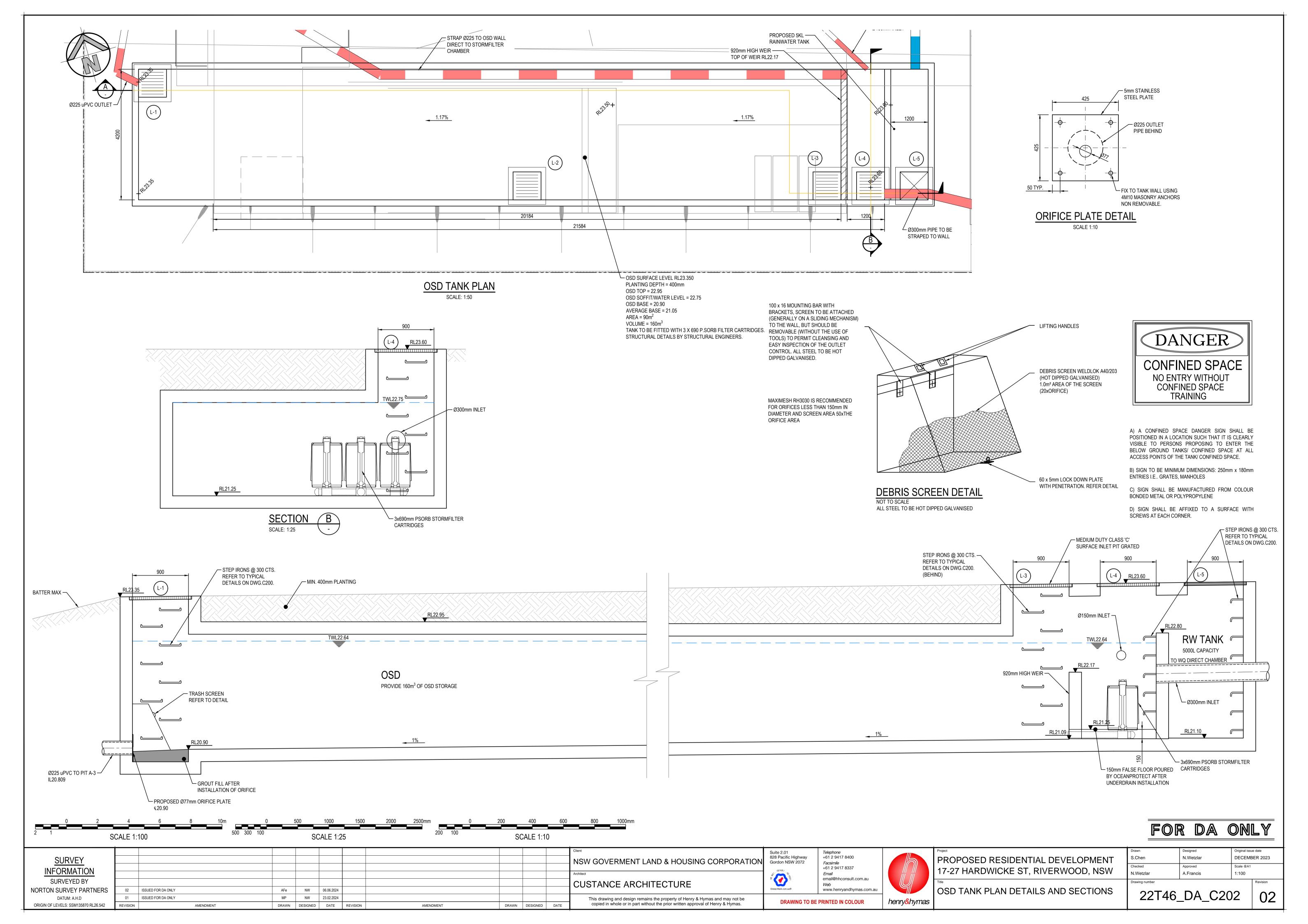
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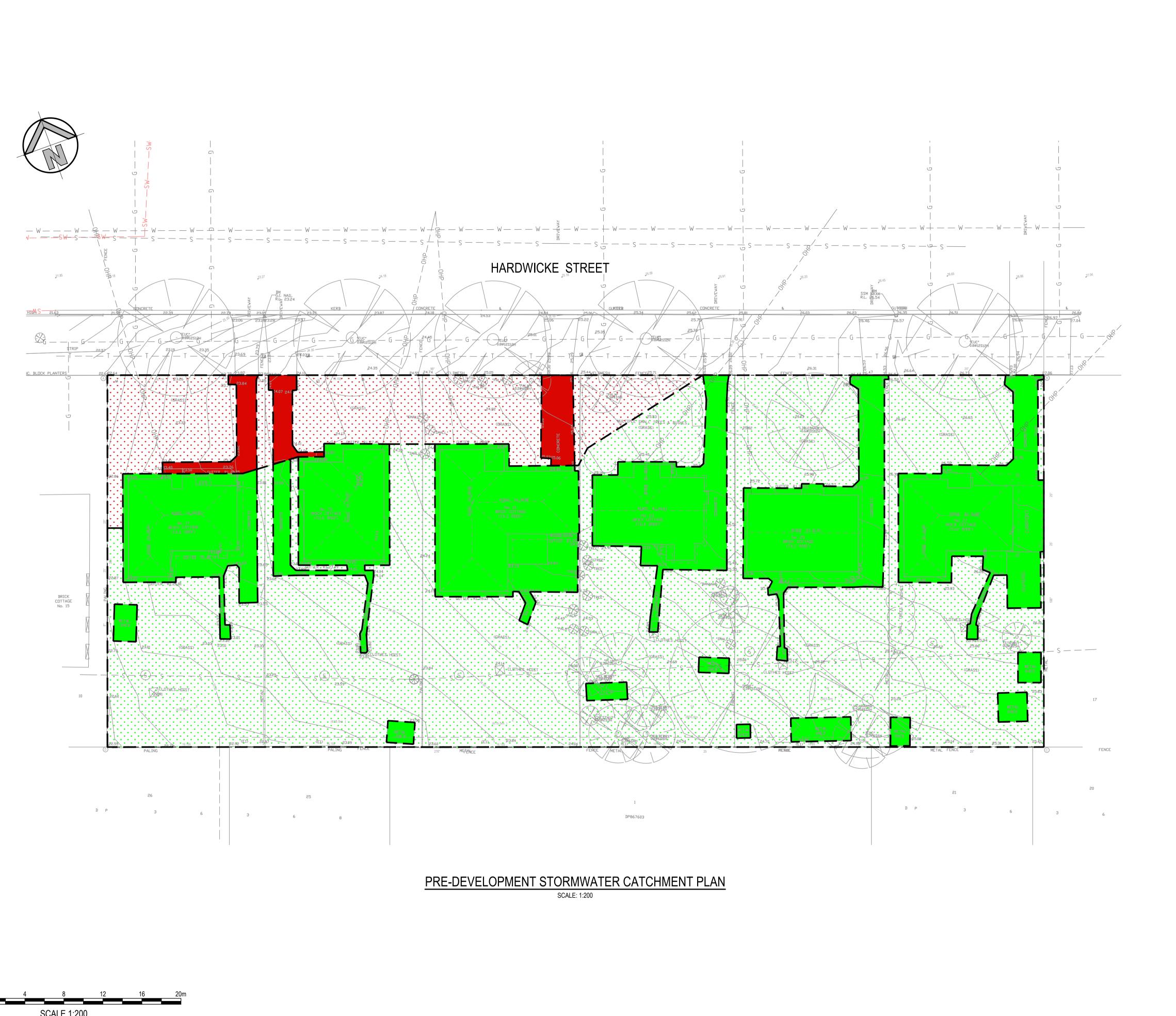
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email@hhconsult.com.au www.henryandhymas.com.au DRAWING TO BE PRINTED IN COLOUR henry&hymas

PROPOSED RESIDENTIAL DEVELOPMENT 17-27 HARDWICKE ST, RIVERWOOD, NSW STORMWATER MISCELLANEOUS DETAILS S.Chen N.Wetzlar DECEMBER 2023 Scale @A1 N.Wetzlar A.Francis 1:200

22T46_DA_C201





TOTAL SITE =3658m²

NORTH CATCHMENT AREA = 81m² (100% IMPERVIOUS)

NORTH CATCHMENT AREA = 394m² (100% PERVIOUS)



SOUTH CATCHMENT AREA = 1124m² (100% IMPERVIOUS)



SOUTH CATCHMENT AREA = 2059m² (100% PERVIOUS)

PRE-DEVELOPMENT FLOW RATES

ARI (YEAR)	CATCHMENT				
	FLOW RATE NORTH (L/s)	FLOW RATE SOUTH (L/s)			
5	13	59			
10	16	73			
20	17	85			
100	20	115			

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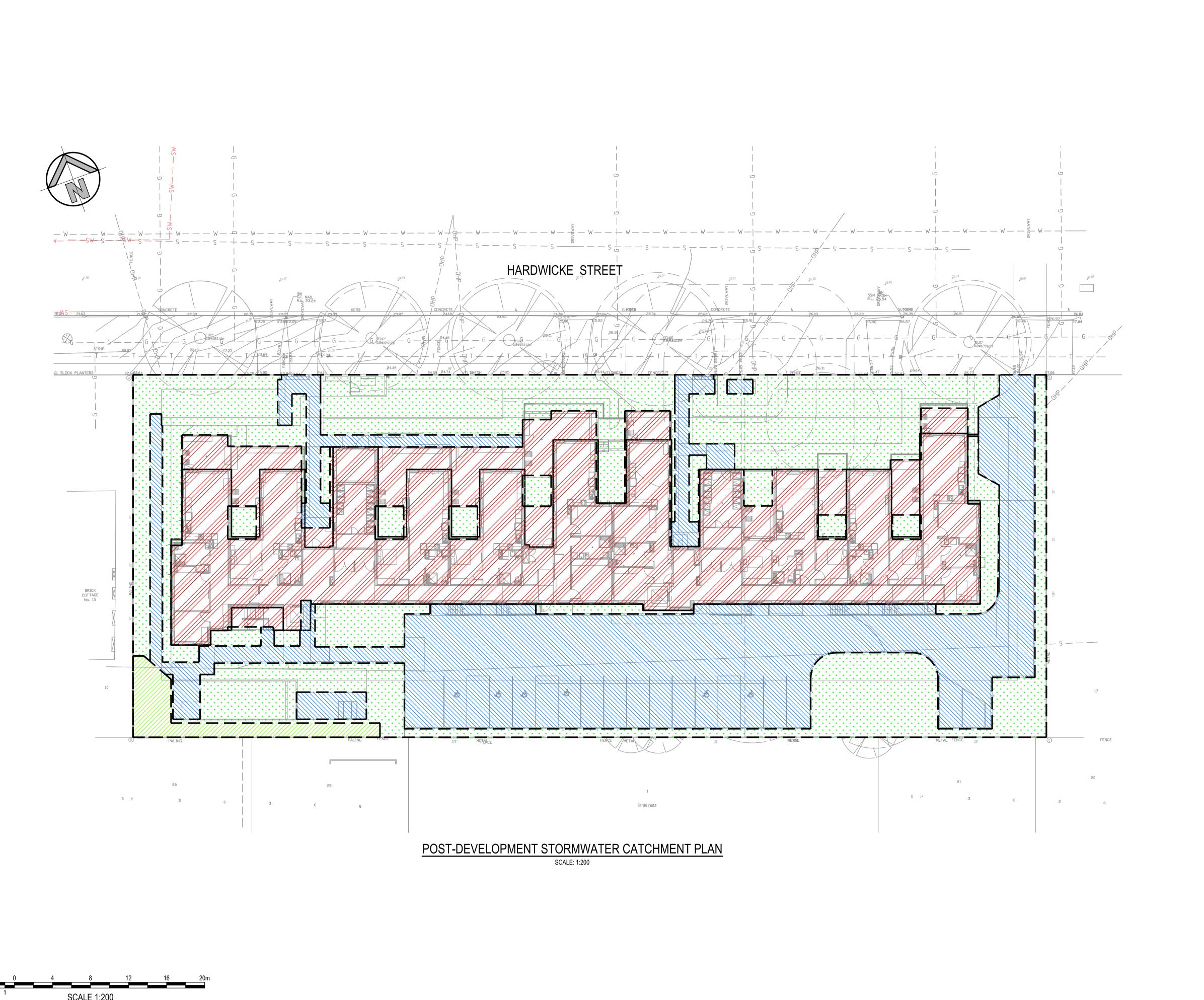
50	JALE I	200									
<u>SURVEY</u>											Client
INFORMATION											NSW GOVERMENT LAND & HOUSING CORPORATION
SURVEYED BY											Architect
NORTON SURVEY PARTNERS	02	ISSUED FOR DA ONLY	PS	NW	06.06.2024						CUSTANCE ARCHITECTURE
DATUM: A.H.D ORIGIN OF LEVELS: SSM135870 RL26.542	01	ISSUED FOR DA ONLY	MP	NW	23.02.2024						This drawing and design remains the property of Henry & Hymas and may not be
ONIGIN OF ELVELS. 33W133070 RE20.342	REVISION	AMENDMENT	DRAWN	DESIGNED	DATE	REVISION	AMENDMENT	DRAWN	DESIGNED	DATE	copied in whole or in part without the prior written approval of Henry & Hymas.

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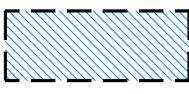
email@hhconsult.com.au www.henryandhymas.com.au

PRE-DEVELOPMENT STORMWATER henry&hymas | CATCHMENT PLAN

PROPOSED RESIDENTIAL DEVELOPMENT N.Wetzlar APRIL 2023 17-27 HARDWICKE ST, RIVERWOOD, NSW 1:200 22T46_DA_C250



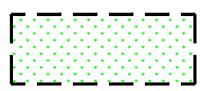
TOTAL SITE =3658m²



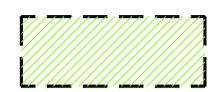
HARDSTAND CATCHMENT $AREA = 1064m^2$



ROOF CATCHMENT AREA = 1308m²



LANDSCAPE CATCHMENT AREA = 1222m²



AREA DRAINING SOUTH AREA = 63.5m²

POST-DEVELOPMENT FLOW RATES

ARI (YEAR)	CATCHMENT				
	FLOW RATE NORTH (DETAINED) (L/s)	FLOW RATE SOUTH (UNDETAINED) (L/s)			
5	12	0			
10	13	0			
20	14	0			
100	17	0			

FOR DA ONLY

	JALL 1.	200									
SURVEY											Client
INFORMATION											NSW GOVERMENT LAND & HOUSING CORPORATION
SURVEYED BY											Architect
NORTON SURVEY PARTNERS											CUSTANCE ARCHITECTURE
DATUM: A.H.D	02	ISSUED FOR DA ONLY	PS	NW	06.06.2024						
ORIGIN OF LEVELS: SSM135870 RL26.542	01	ISSUED FOR DA ONLY	MP	NW	23.02.2024						This drawing and design remains the property of Henry & Hymas and may not be
ORIGIN OF LEVELS. 33M 133070 RE20.342	REVISION	AMENDMENT	DRAWN	DESIGNED	DATE	REVISION	AMENDMENT	DRAWN	DESIGNED	DATE	copied in whole or in part without the prior written approval of Henry & Hymas.

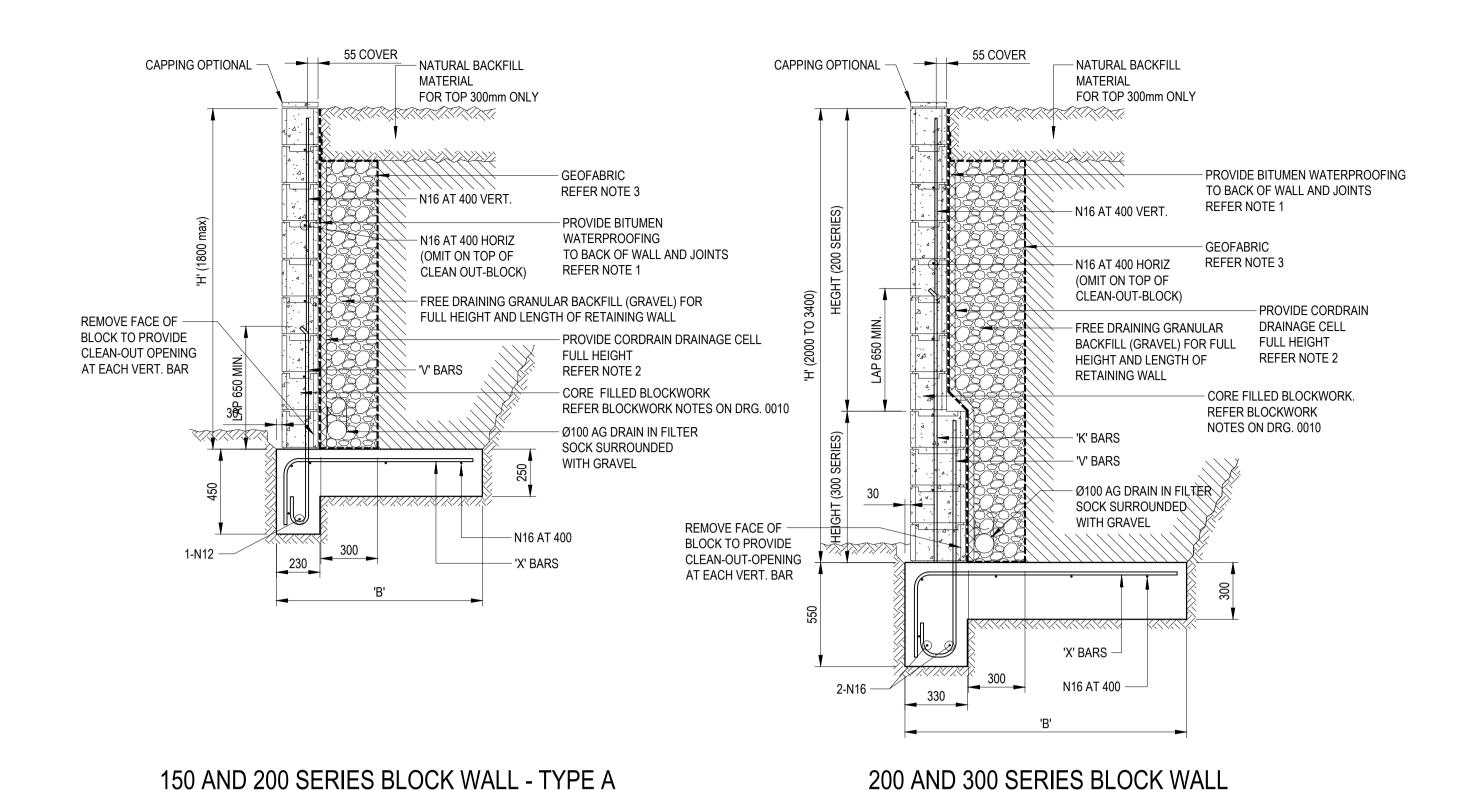
Suite 2.01 828 Pacific Highway Gordon NSW 2072 *Telephone* +61 2 9417 8400 Facsimile +61 2 9417 8337 Soldon So email@hhconsult.com.au

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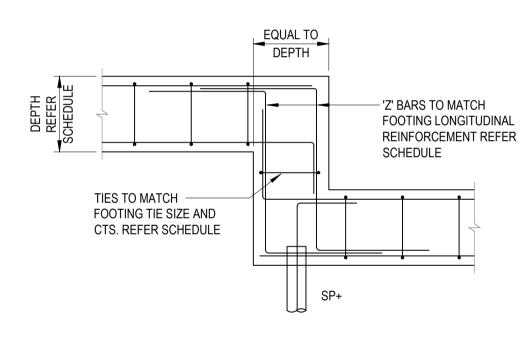
www.henryandhymas.com.au henry&hymas | CATCHMENT PLAN

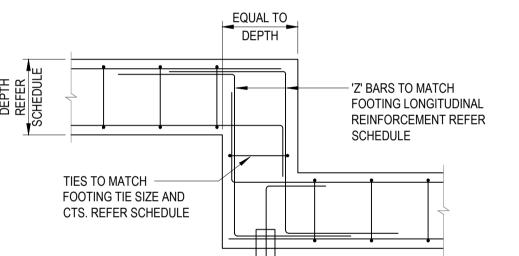
Project
PROPOSED RESIDENTIAL DEVELOPMEN
17-27 HARDWICKE ST, RIVERWOOD, NS
Title
POST-DEVELOPMENT STORMWATER

N.Wetzlar APRIL 2023 Scale @A1 N.Wetzlar A.Francis 1:200 22T46_DA_C251



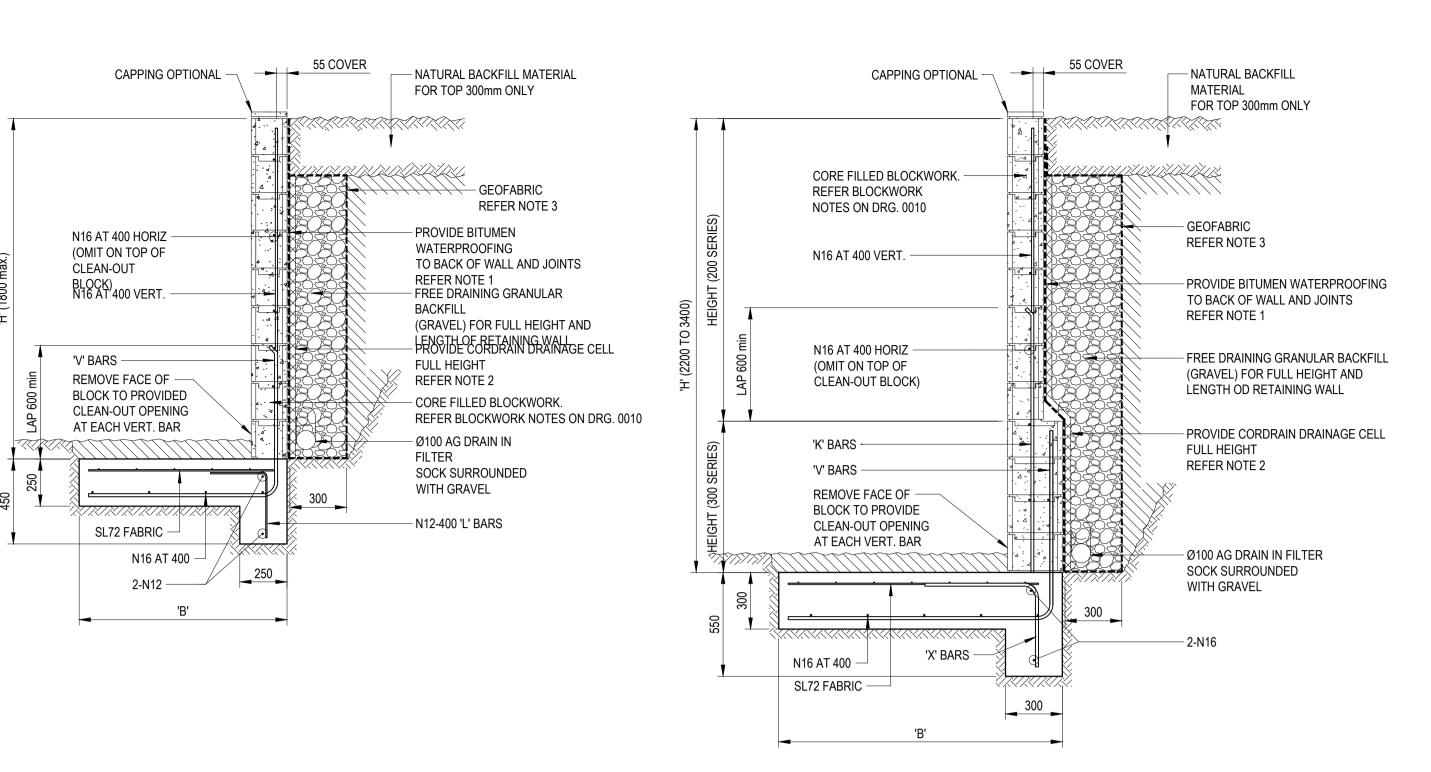
'K' BARS





TYPICAL FOOTING STEP DETAIL

SCALE 1:20



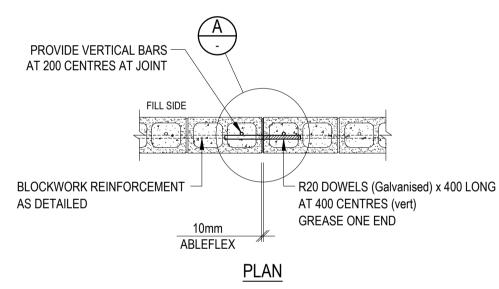
150 OR 200 SERIES BLOCK WALL - TYPE B

200 OR 300 SERIES BLOCK WALL

	RETAINING WALL SCHEDULE - TYPE B								
TOTAL HEIGHT	HEIGHT OF E	BLOCK TYPE	'B' (mm)	'V' & 'X' BARS	'K' BARS				
'H' (mm)	200 SERIES	300 SERIES	D (IIIII)	V & A BANS	IN DAINS				
800	800	-	800	N16 AT 400	-				
1000	1000	-	1200	N16 AT 400	-				
1200	1200	-	1500	N16 AT 400	-				
1400	1400	-	1700	N16 AT 400	-				
1600	1600	-	1900	N16 AT 400	-				
1800	1800	-	2200	N20 AT 400	•				

NOTES:

- 1. ENSURE REAR FACE OF RETAINING WALL IS FULLY WATERPROOFED. USE EMER-PROOF ECOFLEX OE APPROVE EQUIVALENT IN ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS. EG: 2 COATS min. APPLIED IN OPPOSITE DIRECTIONS AND ALLOWED TO CURE FOR 7 DAYS.
- 2. INSTALL FULL HEIGHT DRAINAGE CELL TO REAR OF WALL, USE NYLEX CORDRAIN/18 OR APPROVED EQUIVALENTS.
- 3. PROVIDE GEOFABRIC MATERIAL AS SEPARATION BETWEEN GRANULAR BACKFILL (GRAVEL) AND NATURAL BACKFILL MATERIAL.
- USE BIDIM A24 OR APPROVED EQUIVALENT. 4. MINIMUM ALLOWABLE BEARING PRESSURE AT BASE = 150 kPa



10mm JOINT TO BE KEPT CLEAN FREE OF MORTAR AND MORTAR DROPPINGS ('ABELFLEX' OR SIMILAR JOINT

MASONRY CONSTRUCTION:

EXPOSED ELEMENTS.

TOTAL HEIGHT

800

1000

1200

1400

1600

1800

NOTES:

B1 ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS3700 AND AS NOTED ON THE DRAWINGS.

HEIGHT OF BLOCK TYPE

300 SERIES

200 SERIES

800

1000

1200

1400

1600

1800

(TO BE CONIRMED BY THE ARCHITECT)

USE BIDIM A24 OR APPROVED EQUIVALENT.

4. MINIMUM ALLOWABLE BEARING PRESSURE AT BASE = 150 kPa

- BRICK AND BLOCK COMPRESSIVE STRENGTH (fuc) SHALL BE 15 MPa MINIMUM UNO. STRENGTH GRADE SHALL BE CLEARLY INDICATED ON THE DELIVERY DOCKETS.
- B3 JOINT MORTAR SHALL BE OF CLASS M3 WITH 1:1:6 (CEMENT: LIME: SAND) PROPORTIONS BY VOLUME AND COMPLY WITH AS3700. MORTAR JOINTS SHALL BE 10 mm THICK AND HAVE A MAXIMUM TOOLED DEPTH OF 3 mm UNO. B4 NON-LOAD BEARING WALLS SHALL BE SEPARATED FROM THE LOAD-BEARING
- ELEMENTS BY 15 mm THICK 'CANEITE' OR EXPANDED POLYSTYRENE UNO AT BOTH HORIZONTAL AND VERTICAL FACES. NON-LOAD BEARING WALLS SHALL BE TIED TO THE SOFFITS OF BEAMS OR SLABS OVER BY USING 'MET 4-1' TIES (OR APPROVED EQUIVALENT), AT 800 mm
- MAX. CENTRES, UNO ON THE DRAWINGS, TO MANUFACTURER'S SPECIFICATIONS B5 WHERE CONCRETE SLABS BEAR ON UNREINFORCED MASONRY, INCLUDING CLAY BRICKS, RENDER THE BEARING SURFACE OF THE MASONRY WALL WITH 1:3 (CEMENT : SAND) MORTAR TO ACHIEVE A LEVEL SURFACE AND PLACE A PRE-GREASED METAL SLIP JOINT PROTECTED BY 0.2 mm POLYETHYLENE SHEET

TAPED TO THE FORMWORK BEFORE PLACING CONCRETE. SPECIAL DETAILS

SUCH AS WATER-PROOFING MAY APPLY FOR ROOF SLABS OR SIMILARLY

B6 CONTROL JOINTS

'V' & 'X' BARS

N16 AT 400

N20 AT 400

RETAINING WALL SCHEDULE - TYPE A

900

1000

1100

1200

1300

1400

1. ENSURE REAR FACE OF RETAINING WALL IS FULLY WATERPROOFED. USE EMER-PROOF ECOFLEX OR APPROVED EQUIVALENT IN ACCORDANCE

WITH MANUFACTURERS SPECIFICATION. EG: 2 COATS min. APPLIED IN OPPOSITE DIRECTIONS AND ALLOWED TO CURE FOR 7 DAYS.

2. INSTALL FULL HEIGHT DRAINAGE CELL TO REAR OF WALL, USE NYLEX CORDRAIN/18 OR APPROVED EQUIVALENT, AT 1500 CENTRES.

3. PROVIDE GEOFABRIC MATERIAL AS SEPARATION BETWEEN GRANULAR BACKFILL (GRAVEL) AND NATURAL BACKFILL MATERIAL.

1. CONTROL JOINTS SHALL BE PROVIDED IN MASONRY WALLS AS PER THE TABLE BELOW UNLESS CLOSER SPACINGS ARE SPECIFIED ELSEWHERE IN THE DOCUMENTATION.

MASONRY TYPE	LOCATION	JOINT SIZE (mm)	SPACING (m)
CONCRETE MASONRY	- EXTERNAL	10	7.0
	- EXTERNAL	10	5.0
	(WITH OPENINGS >		
	900mm IN HEIGHT)		
	- INTERNAL (FACE FINISHED)	10	6.0
	- INTERNAL (RENDERED)	10	5.0
LIGHT-WEIGHT MASONRY	- INTERNAL / EXTERNAL	10	6.0
CLAY MASONRY	- INTERNAL / EXTERNAL	15	6.0 *
	- PARAPET WALLS	15	4.0

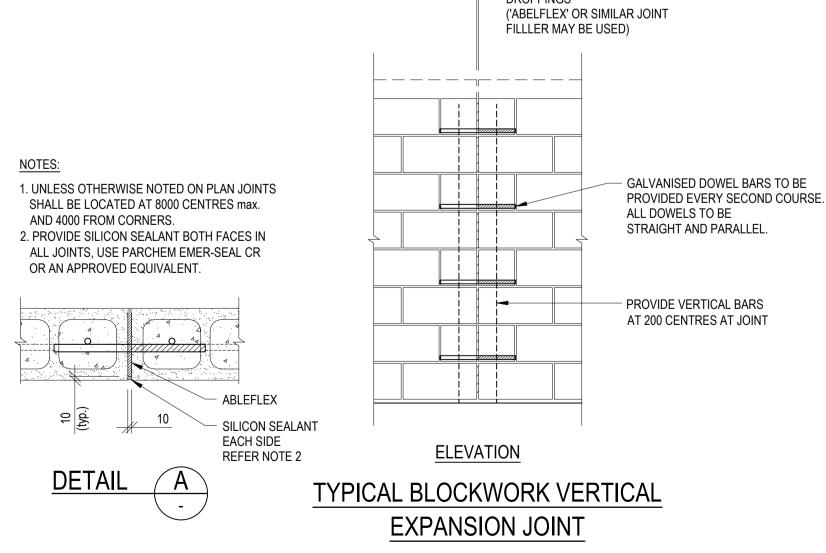
- * FOR REACTIVE 'CLASS M' SITES ONLY. REFER TABLE 4.3 OF AS3700.2011 FOR ARTICULATION JOINTS IN CLAY MASONRY.
- 2. CONTROL JOINTS SHALL BE PLACED AT HALF THE SPECIFIED SPACING FROM A CORNER. PROVIDE JOINTS TO MATCH JOINTS IN THE SUPPORTING STRUCTURE.
- 3. CONTROL JOINTS MUST BE KEPT FREE OF MORTAR AND SEALED WITH A POLYETHYLENE FOAM BACKING ROD SQUEEZED INTO THE GAP AND A GUNNED-IN MASTIC SEALANT. IF THE WALL IS TO BE FIRE-RATED, A FIRE-RATED SEALING SYSTEM WILL BE REQUIRED INSTEAD.

B7 BLOCKWORK

- 1. IN CORE-FILLED BLOCKWORK, EXCESS MORTAR PROTRUDING INTO THE CORES SHALL BE REMOVED BY RODDING AFTER EACH COURSE IS LAID. EVERY CORE FILLED WITH GROUT SHALL HAVE A CLEANOUT BLOCK IN THE BOTTOM COURSE.
- 2. REINFORCEMENT SHALL BE PLACED AND SECURELY TIED IN POSITION AS SHOWN ON THE DRAWINGS. STARTER BARS SHALL BE HELD IN PLACE BY TYING TO HORIZONTAL BARS AT CLEANOUT BLOCKS. PROVIDE COVER TO REINFORCEMENT AS SHOWN IN THE DETAILS.
- 3. CORE FILLING GROUT SHALL BE AS NOTED IN CONCRETE

NOTES IN LIFTS NO MORE THAN 1200mm IN HEIGHT.

Element	Slump	Max. Agg. Size	Cement Type	Exposure Classifn.	Min. Conc. Grade (fc) MPa U.N.O	Conc. Cover (U.N.O.)
Core Filling Grout	230±30	10	GP	-	20	-



EXPANSION JOINT - PLAN & ELEVATION

SCALE 1:20

FOR DA

SCALE 1:20 SURVEY **NSW GOVERMENT LAND & HOUSING CORPORATION INFORMATION** SURVEYED BY **CUSTANCE ARCHITECTURE** NORTON SURVEY PARTNERS 01 ISSUED FOR DA ONLY NW 23.02.2024 DATUM: A.H.D This drawing and design remains the property of Henry & Hymas and may not be copied in whole or in part without the prior written approval of Henry & Hymas. ORIGIN OF LEVELS: SSM135870 RL26.542 DRAWN DESIGNED DATE REVISION AMENDMENT AMENDMENT DRAWN DESIGNED DATE

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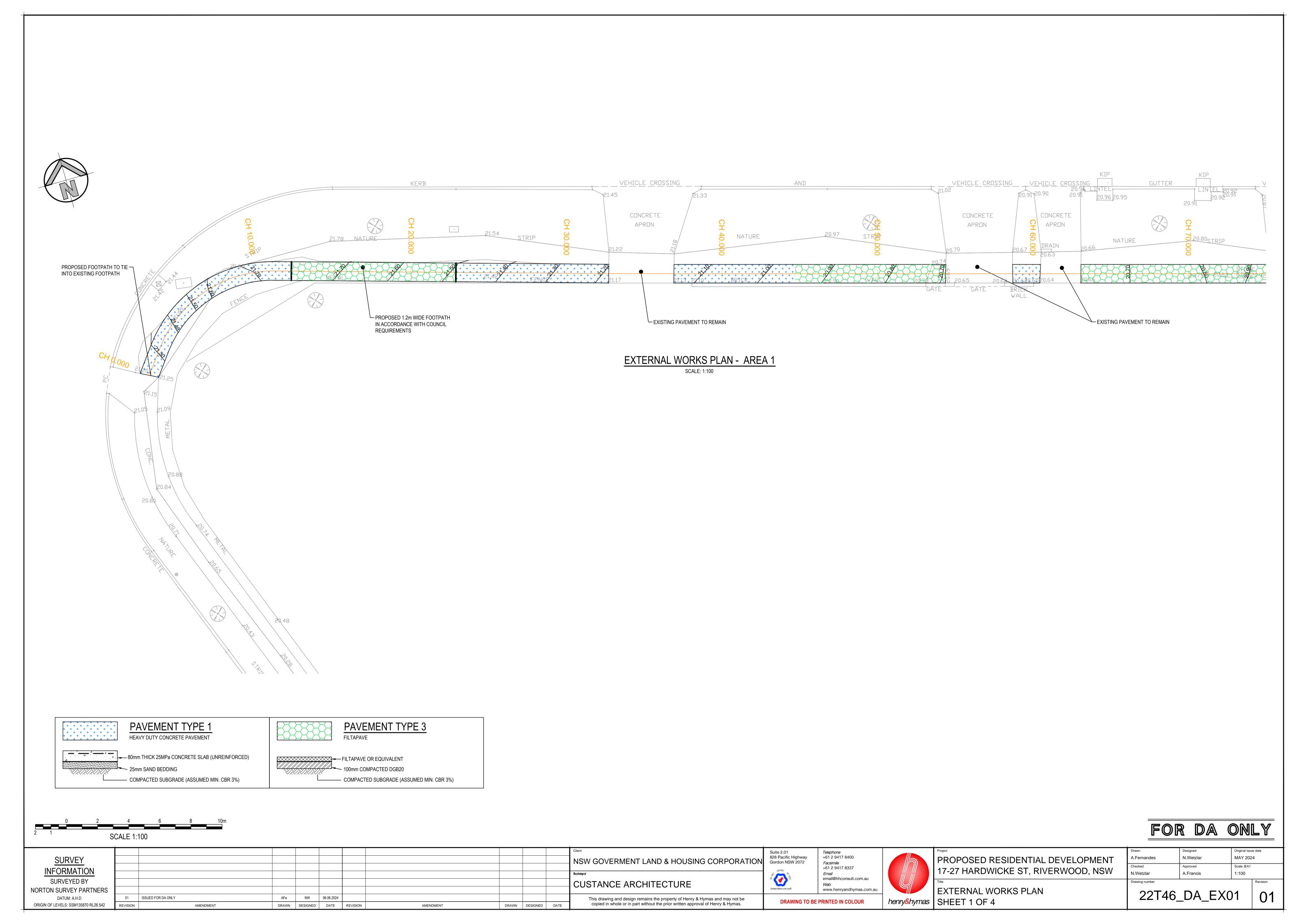
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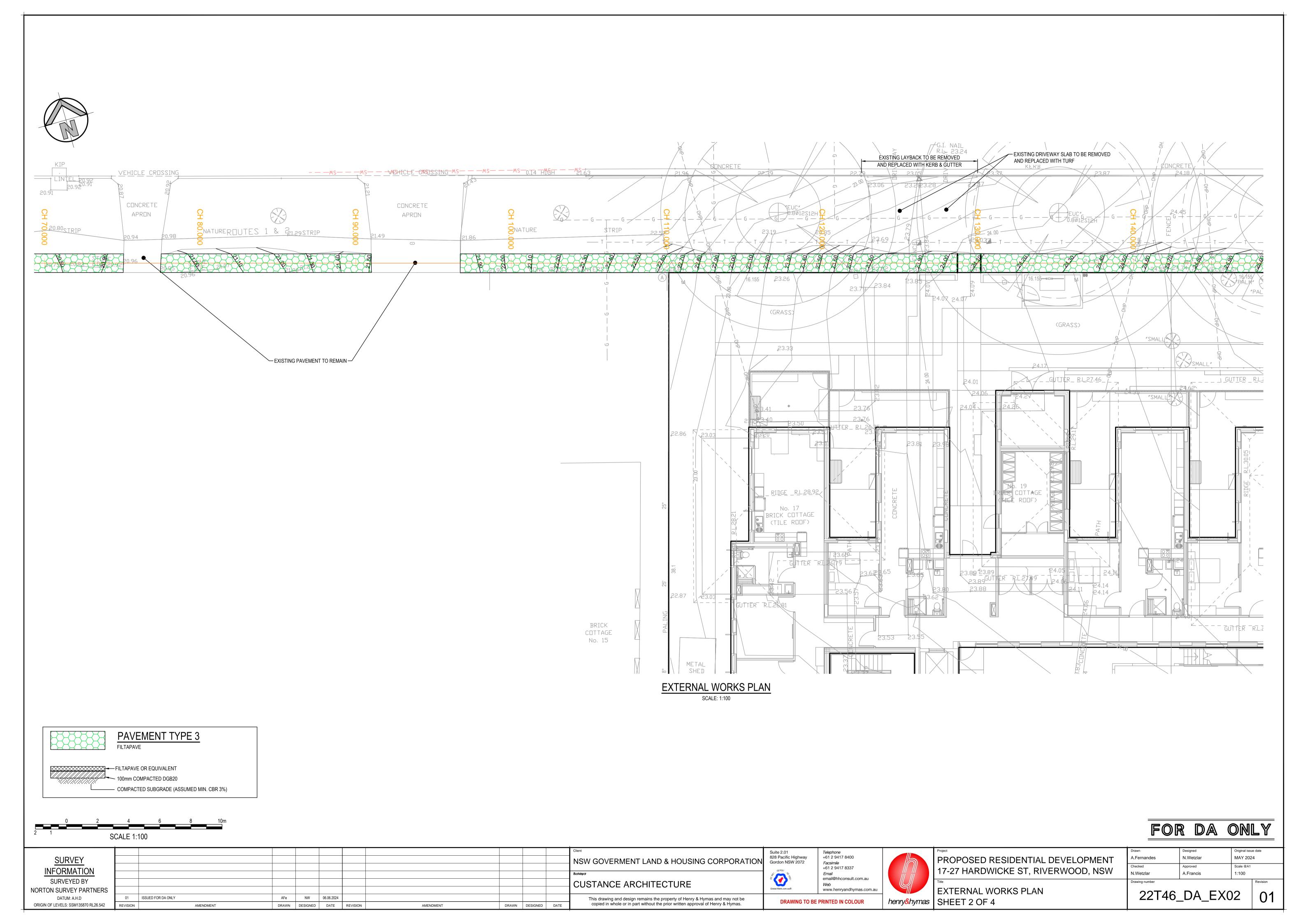
RETAINING WALL DETAILS

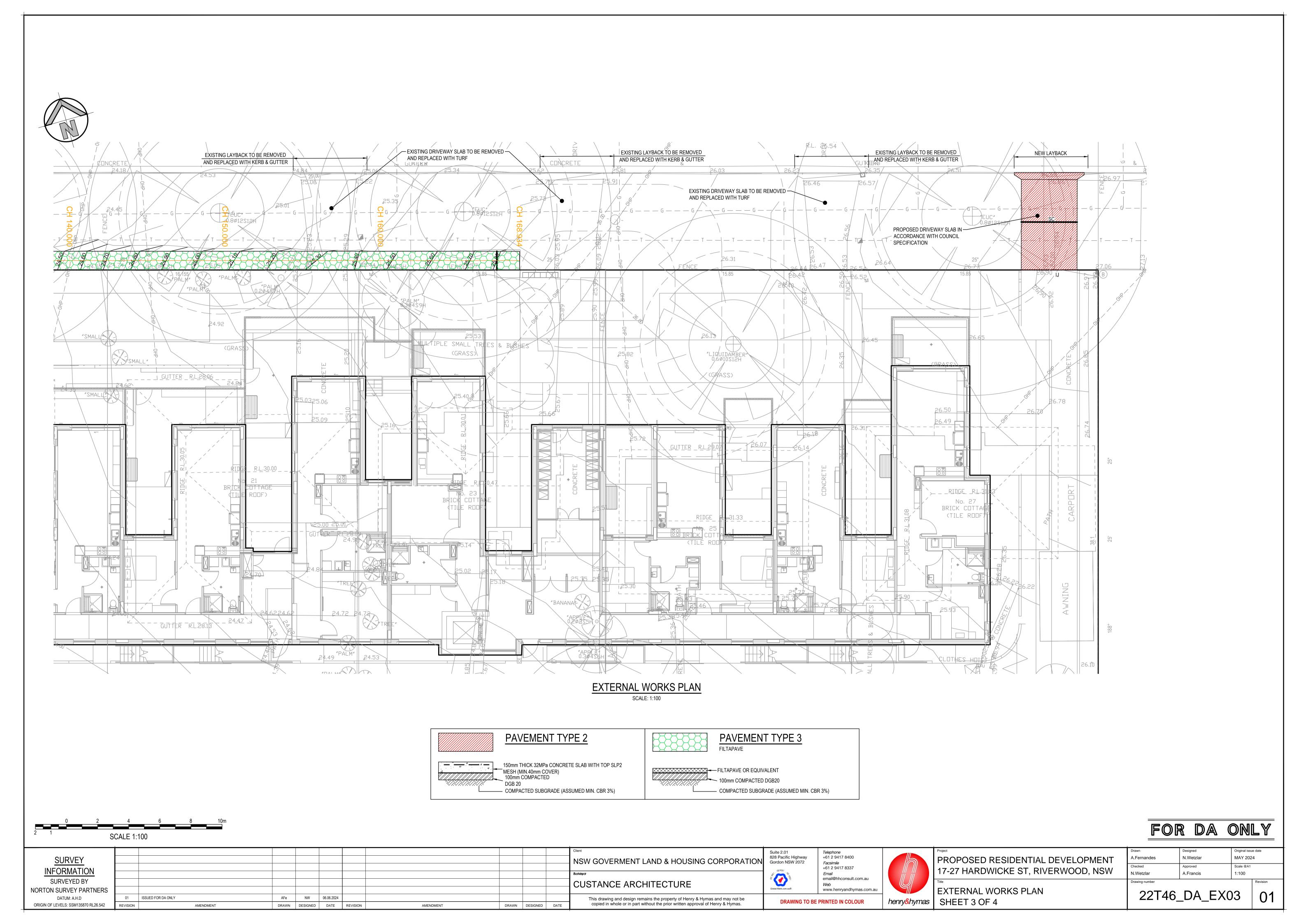
PROPOSED RESIDENTIAL DEVELOPMENT 17-27 HARDWICKE ST, RIVERWOOD, NSW

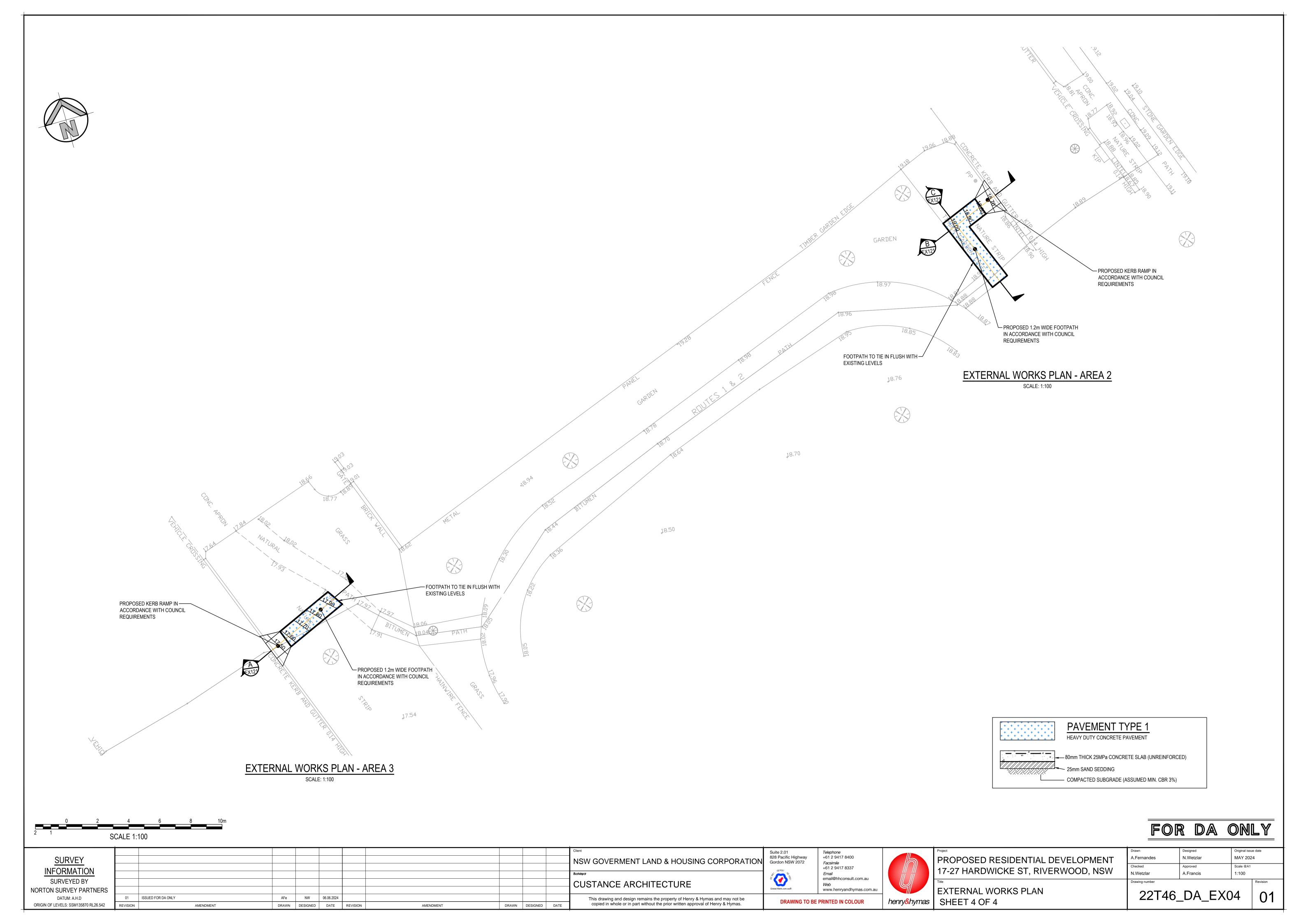
Original issue date S.Chen N.Wetzlar DECEMBER 2023 Scale @A1 N.Wetzlar AS NOTED A.Francis

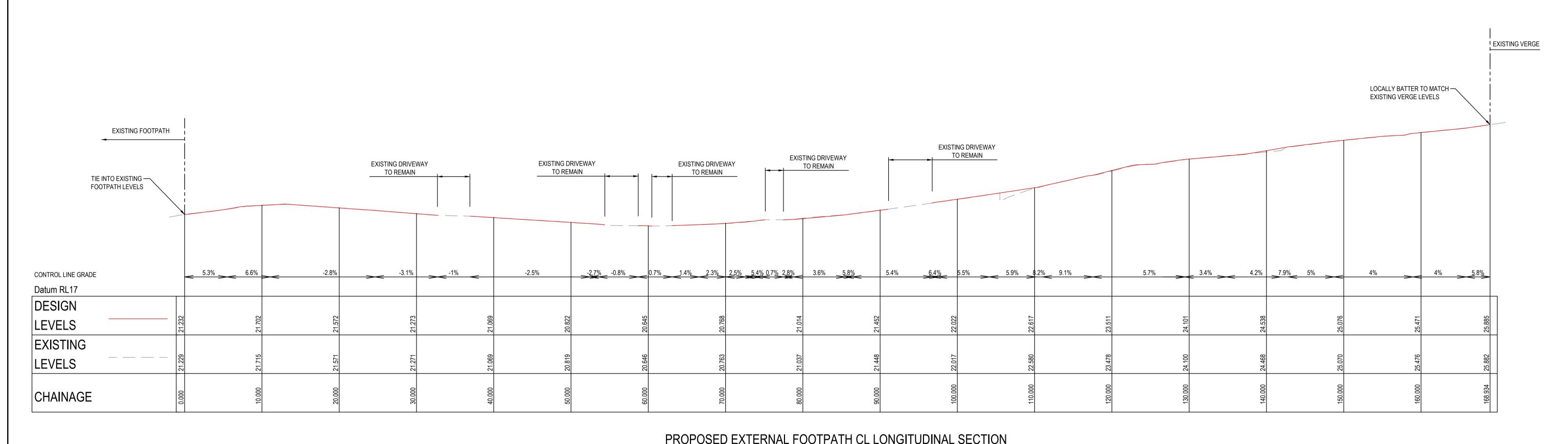
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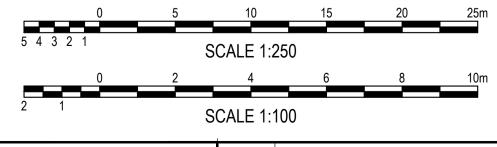






PROPOSED EXTERNAL FOOTPATH CL LONGITUDINAL SECTION

HORIZONTAL SCALE 1:250
VERTICAL SCALE 1:100



FOR DA ONLY

	- O/ (LL 1.	100								
SURVEY										NSW GOVERMENT LAND & HOUSING CORPORATION
<u>INFORMATION</u>										Architect
SURVEYED BY WATSDON BUCHAN										CUSTANCE ARCHITECTURE
DATUM: A.H.D.	01	ISSUED FOR DA ONLY	22	NIA/	06.06.2024		+			
ORIGIN OF LEVELS: SSM 22740 RL41.10	REVISION	AMENDMENT	DRAWN	DESIGNED	06.06.2024 DATE	REVISION AMENDMENT	DRAWN	DESIGNED	DATE	This drawing and design remains the property of Henry & Hymas and may not be copied in whole or in part without the prior written approval of Henry & Hymas.

Telephone	
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+61 2 9417 8337	
Email	
email@hhconsult.com.au	
Web	
www.henryandhymas.com.au	

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Project
PROPOSED RESIDENTIAL DEVELOPMENT
17-27 HARDWICKE ST, RIVERWOOD, NSW
Title
EXTERNAL FOOTPATH LONG SECTION

 Drawn
 Designed
 Original issue date

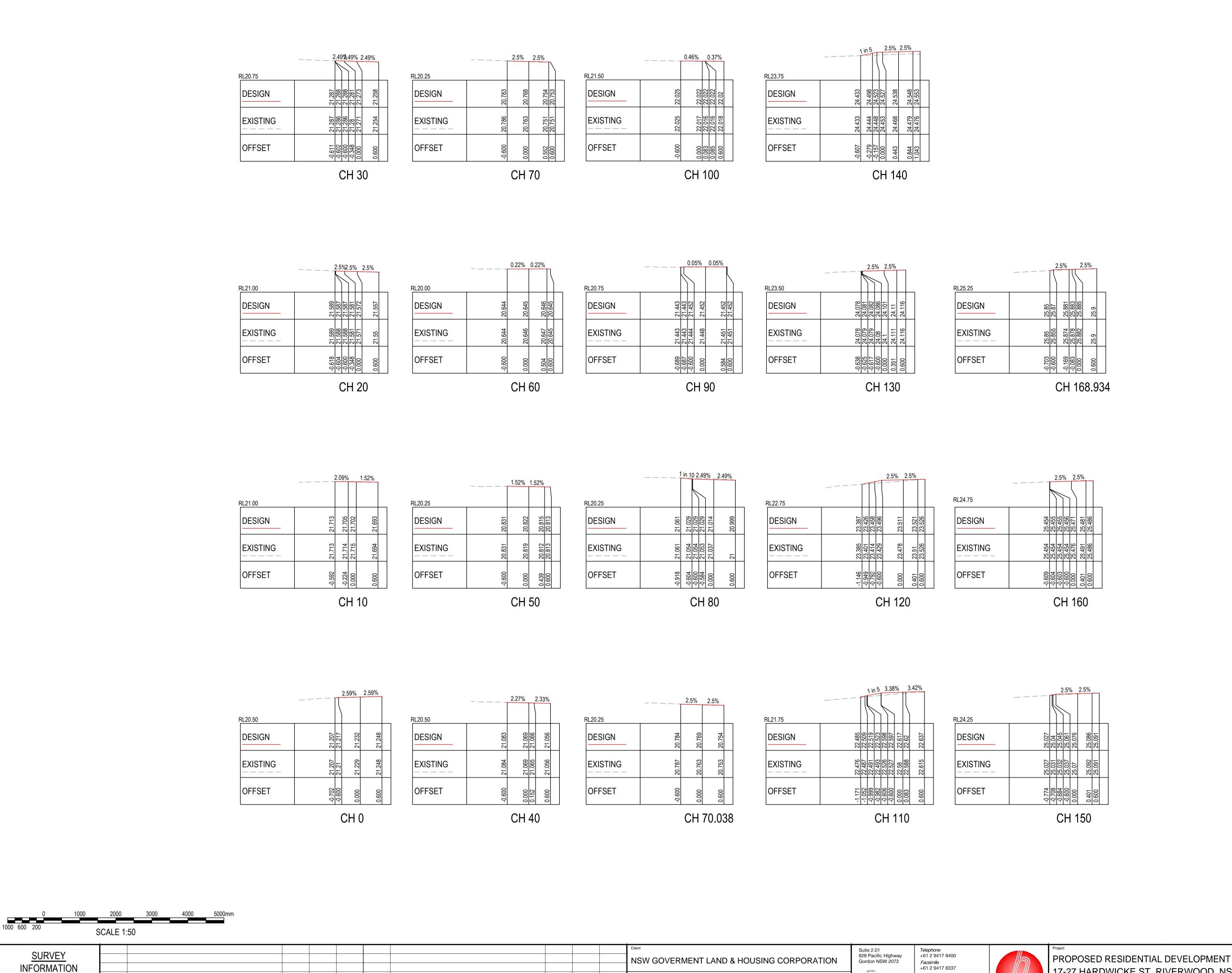
 M.Barrozo
 S.Chen
 FEB 2024

 W
 Checked
 Approved
 Scale @A1

 N.Heazlewood
 A.Francis
 AS NOTED

 Drawing number
 Revision

 ON
 22T46_DA_EX10
 0



DRAWN DESIGNED DATE

SURVEYED BY

WATSDON BUCHAN

DATUM: A.H.D.

ORIGIN OF LEVELS: SSM 22740 RL41.10

01 ISSUED FOR DA ONLY

AMENDMENT

SC NW 06.06.2024

DRAWN DESIGNED DATE REVISION

AMENDMENT

FOR DA ONLY

FEB 2024

Scale @A1

AS NOTED

17-27 HARDWICKE ST, RIVERWOOD, NSW

Title

EXTERNAL FOOTPATH CROSS SECTIONS

72T

Soldword Sol

CUSTANCE ARCHITECTURE

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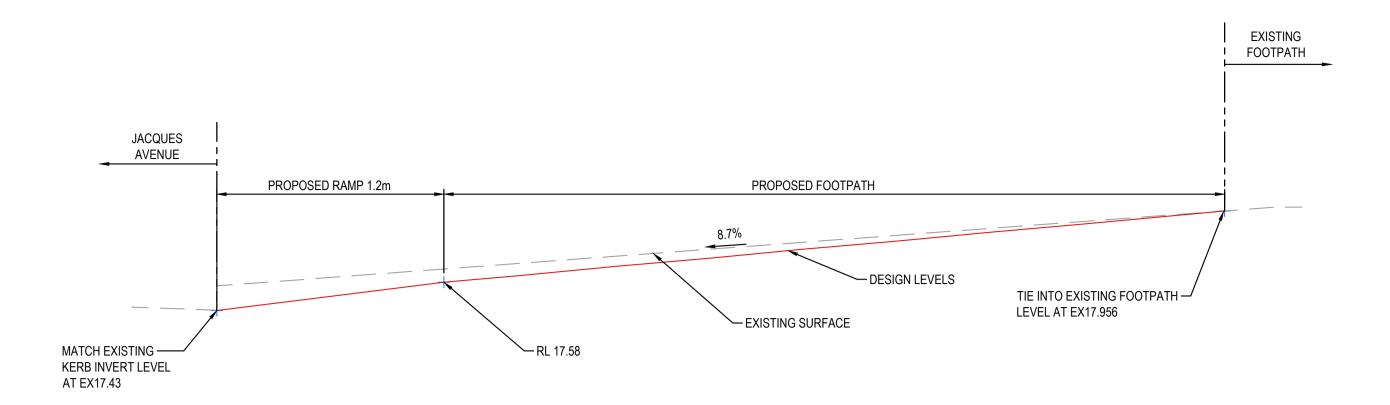
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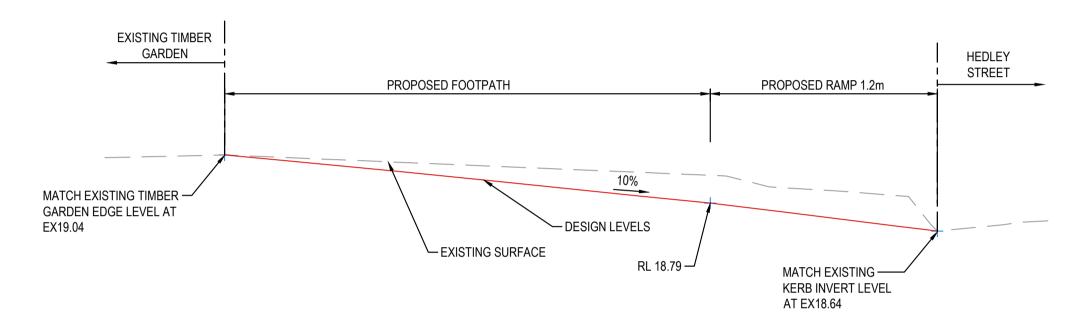
S.Chen

A.Francis

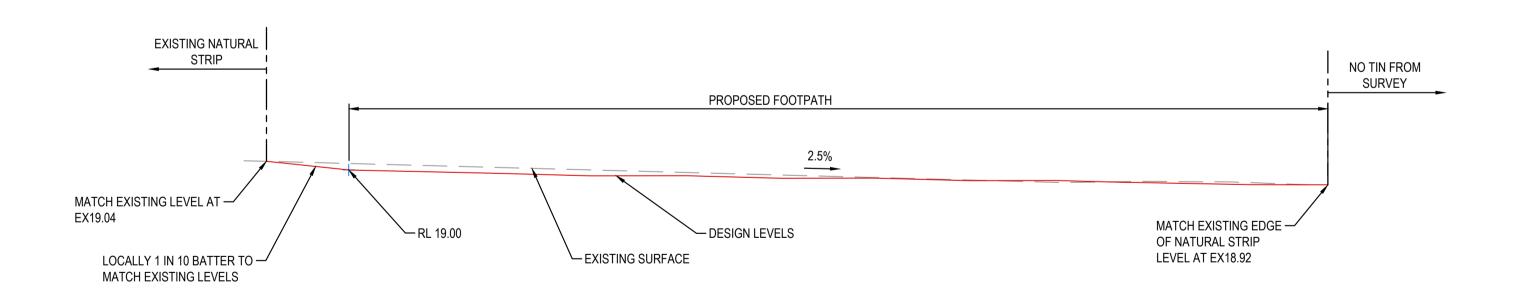
M.Barrozo



EXTERNAL WORKS AREA 3 SECTION - A HORIZONTAL SCALE 1:20 VERTICAL SCALE 1:20



EXTERNAL WORKS AREA 2 SECTION - B HORIZONTAL SCALE 1:20 VERTICAL SCALE 1:20



EXTERNAL WORKS AREA 2 SECTION - C HORIZONTAL SCALE 1:20 VERTICAL SCALE 1:20

SCALE 1:20 **SURVEY** NSW GOVERMENT LAND & HOUSING CORPORATION INFORMATION SURVEYED BY CUSTANCE ARCHITECTURE WATSDON BUCHAN DATUM: A.H.D. 01 ISSUED FOR DA ONLY SC NW 06.06.2024 This drawing and design remains the property of Henry & Hymas and may not be copied in whole or in part without the prior written approval of Henry & Hymas. ORIGIN OF LEVELS: SSM 22740 RL41.10 AMENDMENT DRAWN DESIGNED DATE REVISION AMENDMENT DRAWN DESIGNED DATE

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Signature (Signature)

Telephone +61 2 9417 8400

Facsimile +61 2 9417 8337

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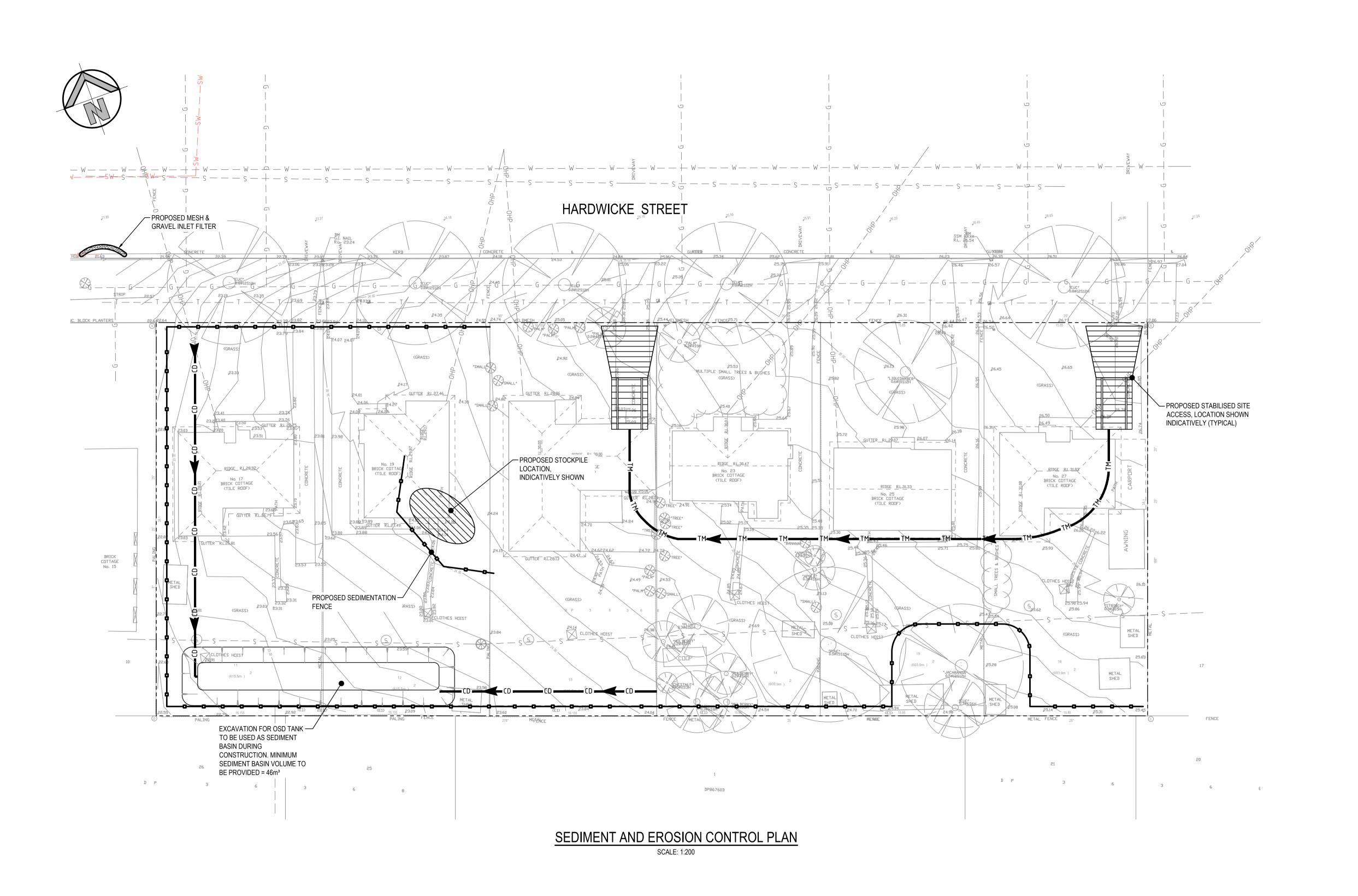
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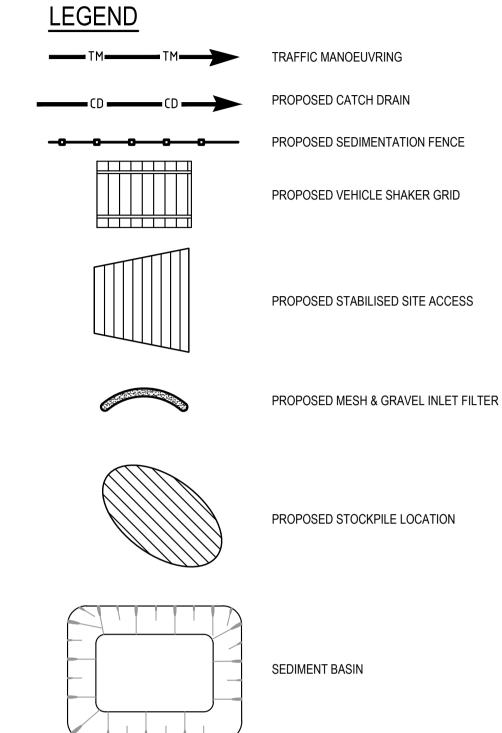
PROPOSED RESIDENTIAL DEVELOPMENT 17-27 HARDWICKE ST, RIVERWOOD, NSW EXTERNAL FOOTPATH RAMP SECTIONS

S.Chen FEB 2024 M.Barrozo Scale @A1 A.Francis AS NOTED N.Heazlewood

22T46_DA_EX12

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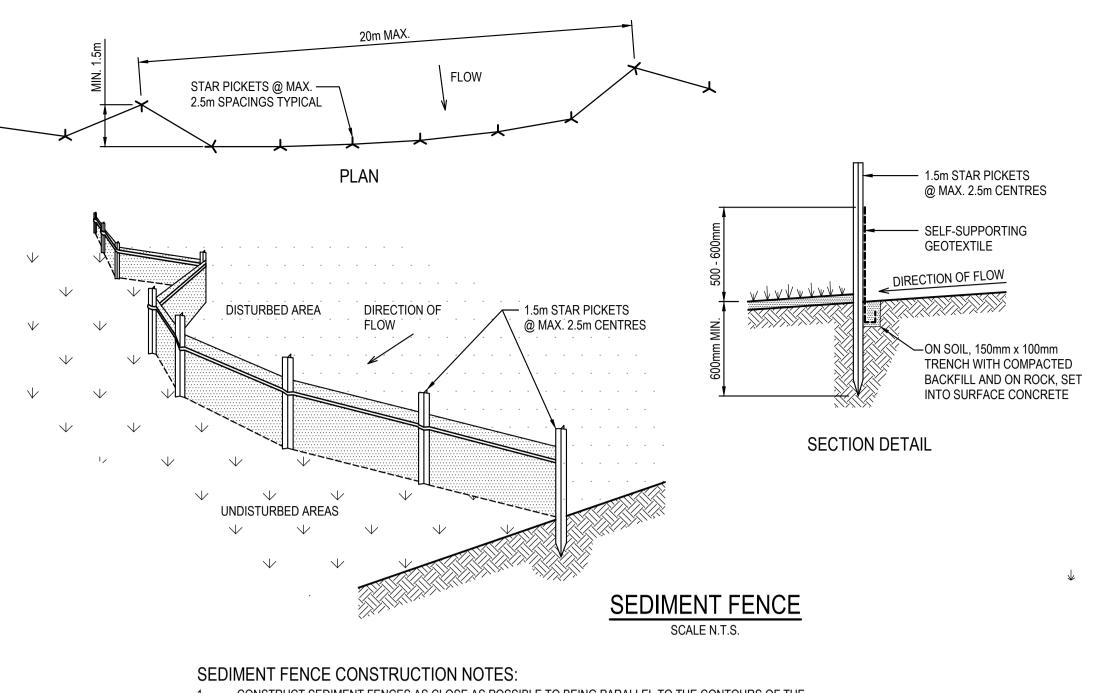


SEDIMENT & EROSION CONTROL NOTES

- ALL SEDIMENT CONTROL DEVICES ARE TO BE CONSTRUCTED, PLACED AND MAINTAINED IN ACCORDANCE WITH RESPECTIVE COUNCIL SPECIFICATIONS AND LANDCOM'S "SOIL AND CONSTRUCTION" MANUAL.
- ALL PERIMETER & SILTATION CONTROL MEASURES ARE TO BE PLACED PRIOR TO, OR AS THE FIRST STEP IN EARTH WORKS AND/OR CLEARING.
- THE SEDIMENT & EROSION CONTROL PLAN MAY REQUIRE FUTURE
 ADJUSTMENT TO REFLECT CONSTRUCTION STAGING. IT IS ALSO THE
 CONTRACTORS RESPONSIBILTY TO PREPARE THEIR OWN SEDIMENT AND
 EROSION CONTROL PLAN WHICH SUITS THE DESIGNED CONSTRUCTION
 STAGING.
- FILTRATION BUFFER ZONES ARE TO BE FENCED OFF AND ACCESS PROHIBITED TO ALL PLANT AND MACHINERY.
- ALL TEMPORARY EARTH BERMS, DIVERSIONS & SILT DAM EMBANKMENTS ARE
 TO BE MACHINE COMPACTED, SEEDED & MULCHED FOR TEMPORARY
 VEGETATION COVER AS SOON AS THEY HAVE BEEN FORMED.
- ALL SEDIMENT TRAPPING STRUCTURES AND DEVICES ARE TO BE INSPECTED
 AFTER STORMS FOR STRUCTURAL DAMAGE OR CLOGGING. TRAPPED MATERIAL
 IS TO BE REMOVED TO A SAFE LOCATION.
- ALL TOPSOIL IS TO BE STOCKPILED ON SITE FOR REUSE (AWAY FROM TREES AND DRAINAGE LINES). MEASURES SHALL BE APPLIED TO PREVENT EROSION OF THE STOCKPILES.
- ALL EARTHWORK AREAS SHALL BE ROLLED EACH EVENING TO SEAL THE EARTHWORKS.
- ALL FILLS ARE TO BE LEFT WITH A LIP AT THE TOP OF THE SLOPE AT THE END.
 ALL CUT AND FILL SLOPES ARE TO BE SEEDED AND STRAW MULCHED WITHIN
 14 DAYS OF COMPLETION OF FORMATION U.N.O. BY LANDSCAPE ARCHITECTS.
- UPON COMPLETION OF ALL EARTHWORKS OR AS DIRECTED BY COUNCIL SOIL CONSERVATION TREATMENTS SHALL BE APPLIED SO AS TO RENDER AREAS THAT HAVE BEEN DISTURBED, EROSION PROOF WITHIN 14 DAYS.
- EROSION AND SILT PROTECTION MEASURES ARE TO BE MAINTAINED AT ALL TIMES.

FOR DA ONLY

4 3 2 1 S	CALE 1	:200																UR UA	
SURVEY												NSW GOVERMENT LAND & HOUSING CORPORATION	Suite 2.01 828 Pacific Highway Gordon NSW 2072	Telephone +61 2 9417 8400 Facsimile		Project PROPOSED RESIDENTIAL DEVELOPMENT	Drawn S.Chen	Designed N.Wetzlar	Original issue date APRIL 2023
INFORMATION												Architect	(2014007)	+61 2 9417 8337 Email email@hhconsult.com.au		17-27 HARDWICKE ST, RIVERWOOD, NSW	Checked N.Wetzlar	Approved A.Francis	Scale @A1 1:200
SURVEYED BY NORTON SURVEY PARTNERS	02	ISSUED FOR DA ONLY		AFe	NW	06.06.2024						CUSTANCE ARCHITECTURE	Global-Mark.com.au®	Web www.henryandhymas.com.au		SEDIMENT AND EROSION CONTROL PLAN	Drawing number	4C DA CE	Revision
DATUM: A.H.D ORIGIN OF LEVELS: SSM135870 RL26.542	01 REVISION	ISSUED FOR DA ONLY	AMENDMENT	MP DRAWN	NW DESIGNED	23.02.2024 DATE	REVISION	AMENDMENT	DRAWN	DESIGNED	DATE	This drawing and design remains the property of Henry & Hymas and may not be copied in whole or in part without the prior written approval of Henry & Hymas.	DRAWING TO	BE PRINTED IN COLOUR	henry&hymas		2212	46_DA_SE	01 02



- 1. CONSTRUCT SEDIMENT FENCES AS CLOSE AS POSSIBLE TO BEING PARALLEL TO THE CONTOURS OF THE SITE, BUT WITH SMALL RETURNS AS SHOWN IN THE DRAWING TO LIMIT THE CATCHMENT AREA OF ANY ONE SECTION. THE CATCHMENT AREA SHOULD BE SMALL ENOUGH TO LIMIT WATER FLOW IF CONCENTRATED AT ONE POINT TO 50 LITRES PER SECOND IN THE DESIGN STORM EVENT, USUALLY THE 10-YEAR EVENT.
- 2. CUT A 150mm DEEP TRENCH ALONG THE UPSLOPE LINE OF THE FENCE FOR THE BOTTOM OF THE FABRIC TO BE ENTRENCHED.
- 3. DRIVE 1.5m LONG STAR PICKETS INTO GROUND @ 2.5m INTERVALS (MAX.) AT THE DOWNSLOPE EDGE OF THE TRENCH. ENSURE ANY STAR PICKETS ARE FITTED WITH SAFETY CAPS.
- 4. FIX SELF-SUPPORTING GEOTEXTILE TO THE UPSLOPE SIDE OF THE POSTS ENSURING IT GOES TO THE BASE OF THE TRENCH. FIX THE GEOTEXTILE WITH WIRE TIES OR AS RECOMMENDED BY THE MANUFACTURER. ONLY USE GEOTEXTILE SPECIFICALLY PRODUCED FOR SEDIMENT FENCING. THE USE OF SHADE CLOTH FOR THIS PURPOSE IS NOT SATISFACTORY.
- 5. JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH A 150mm OVERLAP. 6. BACKFILL THE TRENCH OVER THE BASE OF THE FABRIC AND COMPACT IT THOROUGHLY OVER THE GEOTEXTILE.

— STABILISE STOCKPILE SURFACE

SEDIMENT FENCE —

1. PLACE STOCKPILES MORE THAN 2 (PREFERABLY 5) METRES FROM EXISTING VEGETATION,

STOCKPILES

SCALE N.T.S.

3. WHERE THERE IS SUFFICIENT AREA, TOPSOIL STOCKPILES SHALL BE LESS THAN 2 METRES IN HEIGHT.

4. WHERE THEY ARE TO BE PLACED FOR MORE THAN 10 DAYS, STABILISE FOLLOWING THE APPROVED

5. CONSTRUCT EARTH BANKS ON THE UPSLOPE SIDE TO DIVERT WATER AROUND STOCKPILES AND

EARTH BANK —

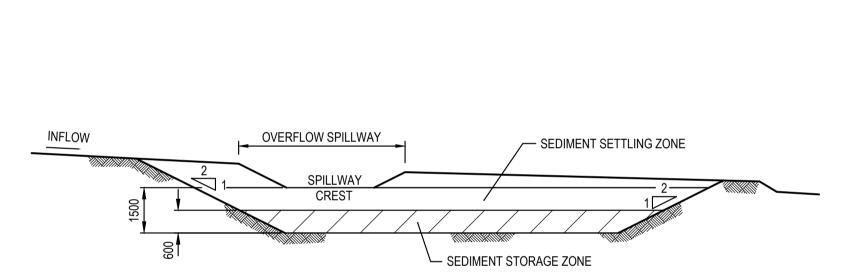
STOCKPILE CONSTRUCTION NOTES:

SEDIMENT FENCES 1 TO 2 METRES DOWNSLOPE.

CONCENTRATED WATER FLOW, ROADS AND HAZARD AREAS.

2. CONSTRUCT ON THE CONTOUR AS LOW, FLAT, ELONGATED MOUNDS.

E.S.C.P. OR S.W.M.P. TO REDUCE THE C-FACTOR TO LESS THAN 0.10.



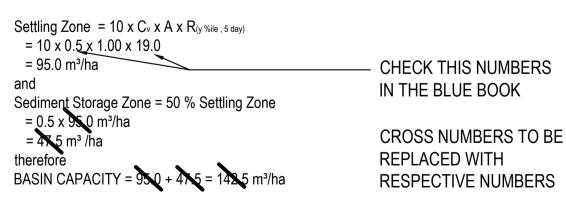
PERSPECTIVE VIEW

TYPE 'D' & 'F' SEDIMENTATION BASIN

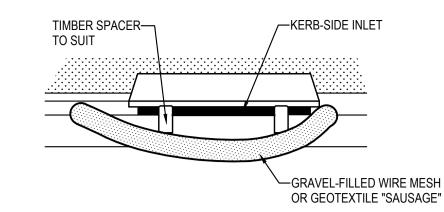
TYPICAL SECTION

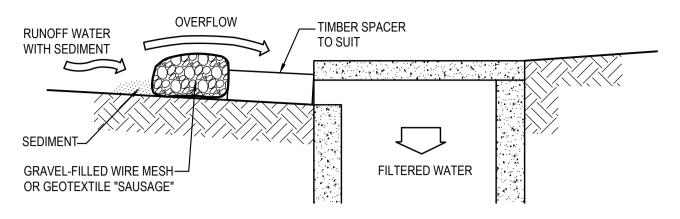
SEDIMENTATION BASIN CALCULATIONS: (FOR TYPE D & F SOILS) - PER 1 ha

BASIN CAPACITY = SETTLING ZONE + SEDIMENT STORAGE ZONE



The basin is to be a slow settling basin with a slow discharge to the level of required storage capacity. This will mean that sediment will settle over a period of approximatly 5 days.



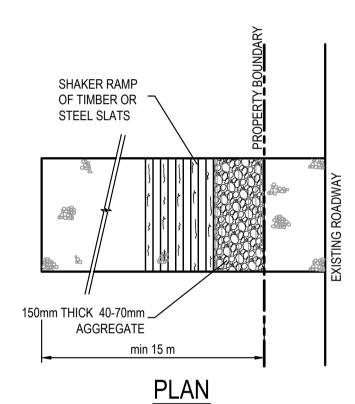


MESH & GRAVEL INLET FILTER CONSTRUCTION NOTES:

- 1. FABRICATE A SLEEVE MADE FROM GEOTEXTILE OR WIRE MESH LONGER THAN THE LENGTH OF THE INLET
- PIT AND FILL IT WITH 25mm TO 50mm GRAVEL.

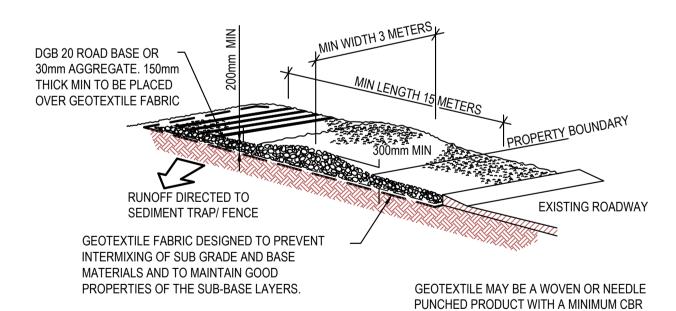
 2. FORM AN ELLIPTICAL CROSS-SECTION ABOUT 150mm HIGH x 400mm WIDE.
- 3. PLACE THE FILTER AT THE OPENING LEAVING AT LEAST A 100mm SPACE BETWEEN IT AND THE KERB INLET.
- MAINTAIN THE OPENING WITH SPACER BLOCKS.
- 4. FORM A SEAL WITH THE KERB TO PREVENT SEDIMENT BYPASSING THE FILTER.
- 5. SANDBAGS FILLED WITH GRAVEL CAN SUBSTITUTE FOR THE MESH OR GEOTEXTILE PROVIDING THEY ARE PLACED SO THAT THEY CAN FIRMLY ABUT EACH OTHER AND SEDIMENT / LADEN WATERS CANNOT PASS RETWEEN

MESH & GRAVEL INLET FILTER SCALE N.T.S.



STABILISED SITE ACCESS WITH SHAKER RAMP

CONSTRUCTION SITE



STABILISED SITE ACCESS WITH SHAKER RAMP

BURST STRENGTH (AS3706.4-90) OF 2500 N

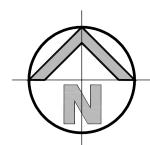
N.T.S.

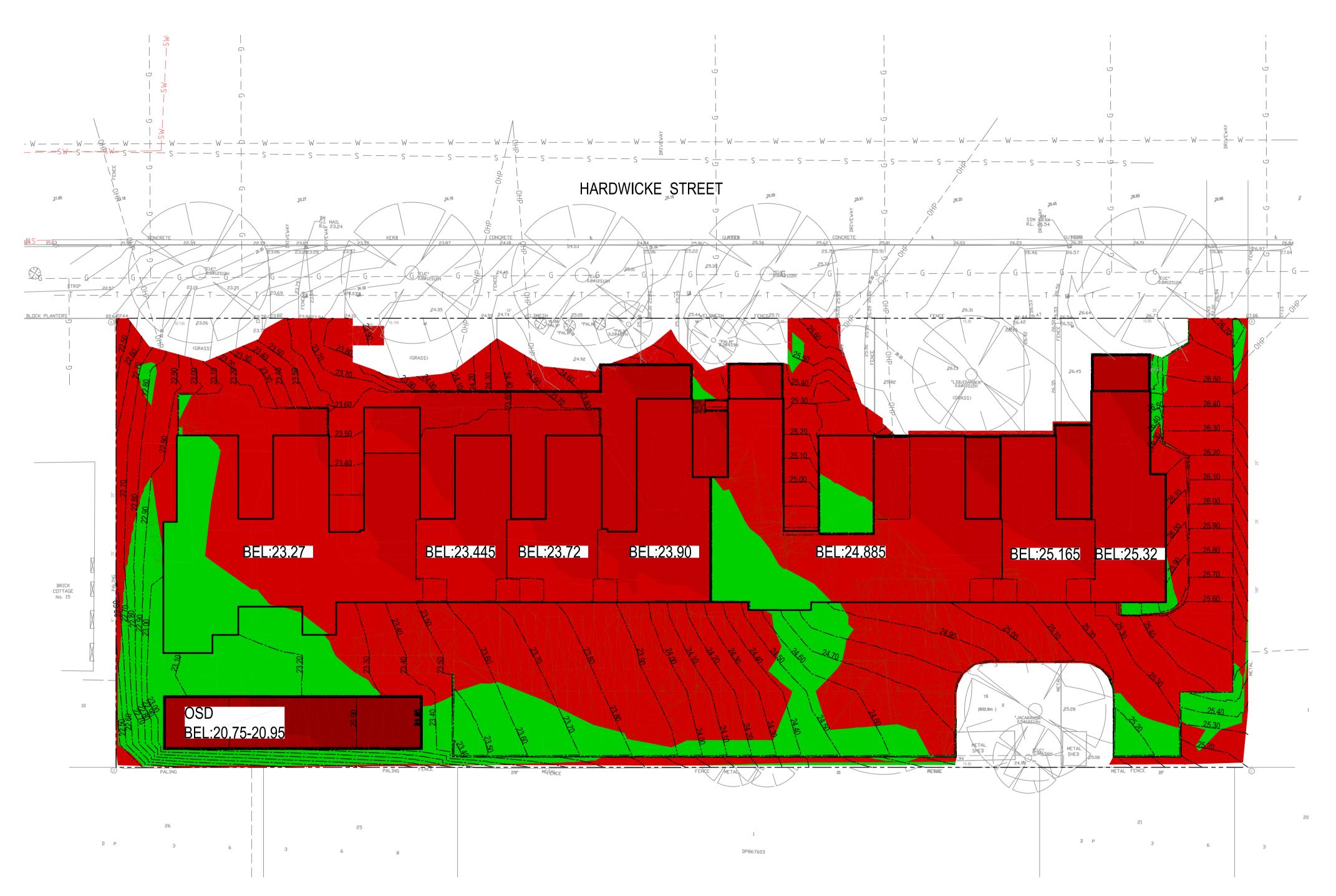
NOTES:

- THIS DEVICE IS TO BE LOCATED AT ALL EXITS FROM CONSTRUCTION SITE.
- THIS DEVICE IS TO BE REGULARLY CLEANED OF DEPOSITED MATERIAL SO AS TO MAINTAIN A 50mm DEEP SPACE BETWEEN PLANKS.
- 3. ANY UNSEALED ROAD BETWEEN THIS DEVICE AND NEAREST ROADWAY IS TO BE TOPPED WITH 100mm THICK 40-70mm SIZE AGGREGATE.
- 4. ALTERNATIVELY, THREE(3) PRECAST CONCRETE CATTLE GRIDS (AS MANUFACTURED BY "HUMES CONCRETE MAY BE USED. 1, 2 & 3 ABOVE ALSO APPLY.

FOR DA ONLY

									Client	Suite 2.01	Telephone		Project	Drawn	Designed	Original issue date
SURVEY									NSW GOVERMENT LAND & HOUSING CORPORATION	828 Pacific Highway Gordon NSW 2072	Telephone +61 2 9417 8400 Facsimile		PROPOSED RESIDENTIAL DEVELOPMENT	S.Chen	N.Wetzlar	APRIL 2023
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<u>INFORMATION</u>									Architect	(3) 1400) E	Email		17-27 HARDWICKE ST, RIVERWOOD, NSW	N.Wetzlar	A.Francis	NTS
SURVEYED BY									CUSTANCE ARCHITECTURE	og Man	email@hhconsult.com.au Web		Title	Drawing number		Revision
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BULK EARTHWORKS CUT AND FILL PLAN SCALE: 1:200

LEGEND

— - - — - - — EXISTING BOUNDARY

DEPTH R		Upper_v	alue	Colour
-10	to	-7	m	
-7	to	-6	m	
-6	to	-5	m	
-5	to	-4	m	
-4	to	-3	m	
-3	to	-2.5	m	
-2.5	to	-2	m	
-2	to	-1.5	m	
-1.5	to	-1	m	
-1	to	5	m	
5	to	.0	m	
.0	to	.5	m	
.5	to	1	m	
1	to	1.5	m	
1.5	to	2.	m	
2	to	2.5	m	
2.5	to	3.	m	
3.	to	4	m	
4	to	5	m	
5	to	6	m	
6	to	7	m	
7	to	10	m	
10	to	90	m	

EARTHWORKS QUANTITIES

NOT TO BE USED FOR CONTRACTUAL PURPOSES. TENDERERS TO DETERMINE VOLUMES USING THEIR OWN METHOD OF CALCULATION.

- EARTHWORKS QUANTITIES -							
TOTAL AREA (3131 m²)							
CUT	1044 m³						
FILL	75 m³						
EXCESS OF CUT OVER FILL 969 m³							
TOPSOIL STRIPPING "200mm" OF 63	TOPSOIL STRIPPING "200mm" OF 632.2m3 NOT INCLUDED IN CALCULATION						
EXCAVATION FOR SERVICE TRENCHES NOT INCLUDED IN CALCULATION							
EXCAVATION FOR RETAINING WALLS NOT INCLUDED IN CALCULATION							

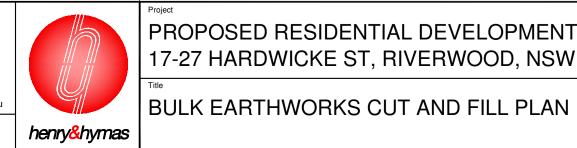
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BUILDING SLAB	280	mm
OTHER AREAS	250	mm

SCALE 1:200

٥	CALE	1.200											
OLIDVEV													Client
SURVEY													NSW GOVERMENT LAND & HOUSING CORPORATION
INFORMATION													Architect
SURVEYED BY													CUSTANCE ARCHITECTURE
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Suite 2.01 828 Pacific Highway **Telephone** +61 2 9417 8400 Gordon NSW 2072 **Facsimile** +61 2 9417 8337 email@hhconsult.com.au www.henryandhymas.com.au

DRAWING TO BE PRINTED IN COLOUR



PROPOSED RESIDENTIAL DEVELOPMENT 17-27 HARDWICKE ST, RIVERWOOD, NSW

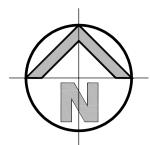
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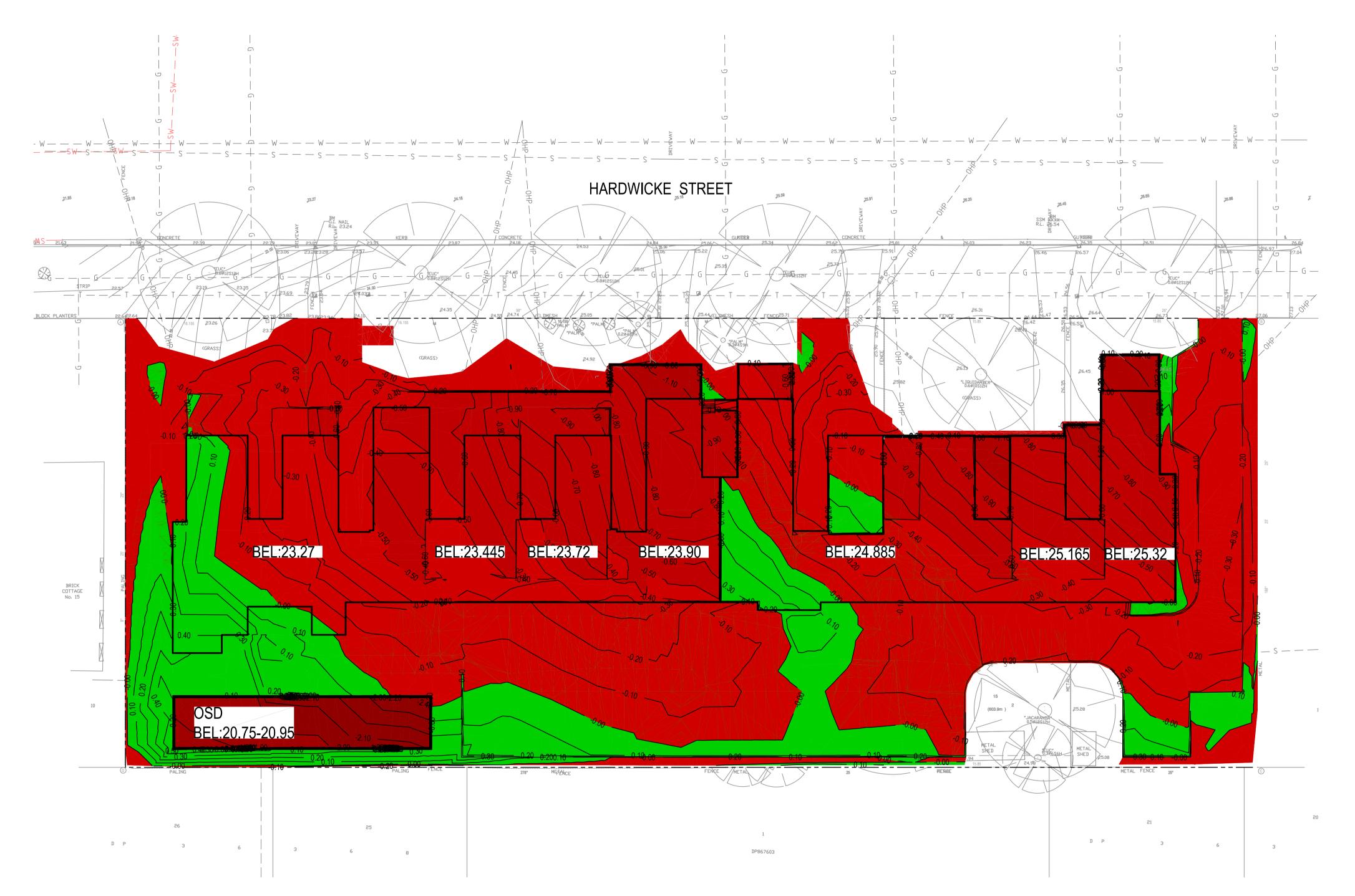
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1:200

22T46_DA_BE01

A.Francis





BULK EARTHWORKS CUT AND FILL DEPTH PLAN SCALE: 1:200

LEGEND

DEPTH Lower_		Upper_v	alue	Colour
-10	to	-7	m	
-7	to	-6	m	
-6	to	-5	m	
-5	to	-4	m	
-4	to	-3	m	
-3	to	-2.5	m	
-2.5	to	-2	m	
-2	to	-1.5	m	
-1.5	to	-1	m	
-1	to	5	m	
5	to	.0	m	
.0	to	.5	m	
.5	to	1	m	
1	to	1.5	m	
1.5	to	2.	m	
2	to	2.5	m	
2.5	to	3.	m	
3.	to	4	m	
4	to	5	m	
5	to	6	m	
6	to	7	m	
7	to	10	m	
10	to	90	m	

EARTHWORKS QUANTITIES

NOT TO BE USED FOR CONTRACTUAL PURPOSES. TENDERERS TO DETERMINE VOLUMES USING THEIR OWN METHOD OF CALCULATION.

- EARTHWORKS QUANTITIES -						
TOTAL AREA (3161 m²)						
CUT	1044 m³					
FILL	75 m³					
EXCESS OF CUT OVER FILL 969 m³						
TOPSOIL STRIPPING "200mm" OF 63	TOPSOIL STRIPPING "200mm" OF 632.2m3 NOT INCLUDED IN CALCULATION					
EXCAVATION FOR SERVICE TRENCHES NOT INCLUDED IN CALCULATION						
EXCAVATION FOR RETAINING WALLS NOT INCLUDED IN CALCULATION						

PAVEMENT THICKNESS "INCLUDING BED	DING THICKN	IESS"
BUILDING SLAB	280	mm
OTHER AREAS	250	mm



5	CALE	1:200										
SURVEY											NSW GOVERMENT LAND & HOUSING CORPORATION	8
INFORMATION SURVEYED BY NORTON SURVEY PARTNERS	02	ISSUED FOR DA ONLY		SC	NH	06.06.2024					CUSTANCE ARCHITECTURE	<u>\$</u>
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DRAWING TO BE PRINTED IN COLOUR



Project
PROPOSED RESIDENTIAL DEVELOPMENT
17-27 HARDWICKE ST, RIVERWOOD, NSW

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Drawn	Designed	Original issu	ie date
S.Chen	N.Wetzlar	DECEME	BER 2023
Checked	Approved	Scale @A1	
N.Wetzlar	A.Francis	1:200	
Drawing number			Revision

FOR DA ONLY

BULK EARTHWORKS CUT

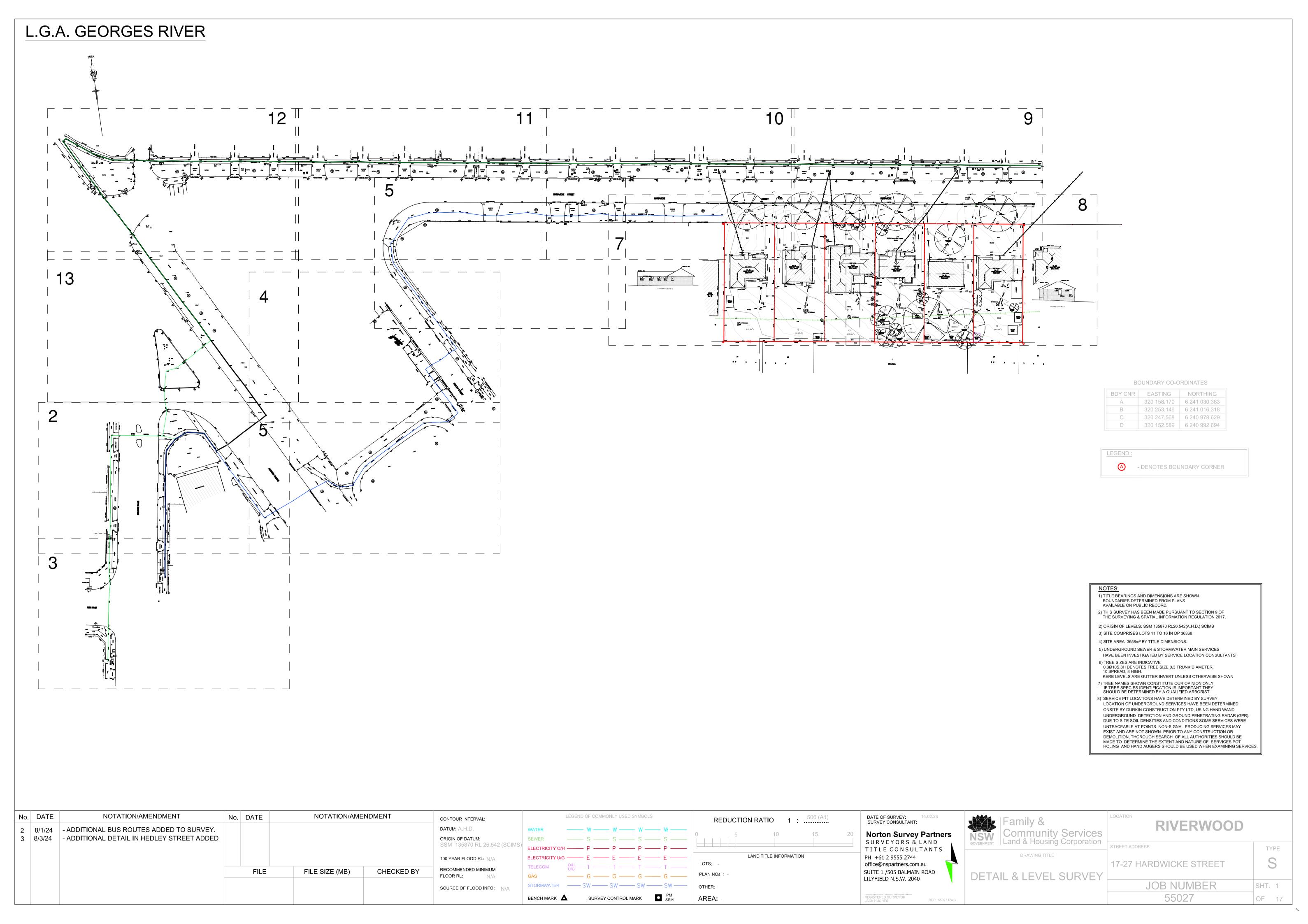
henry&hymas AND FILL DEPTH PLAN

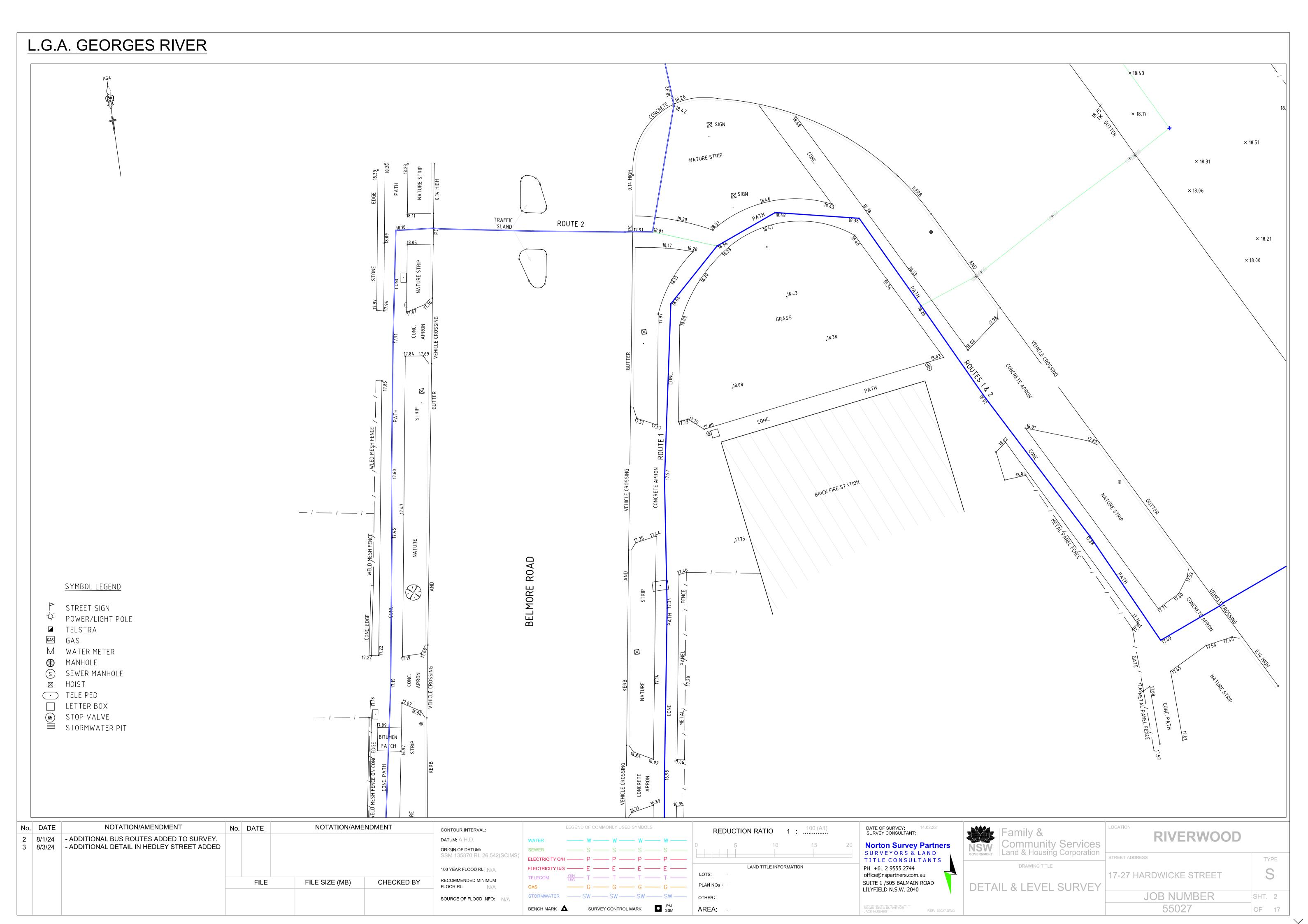
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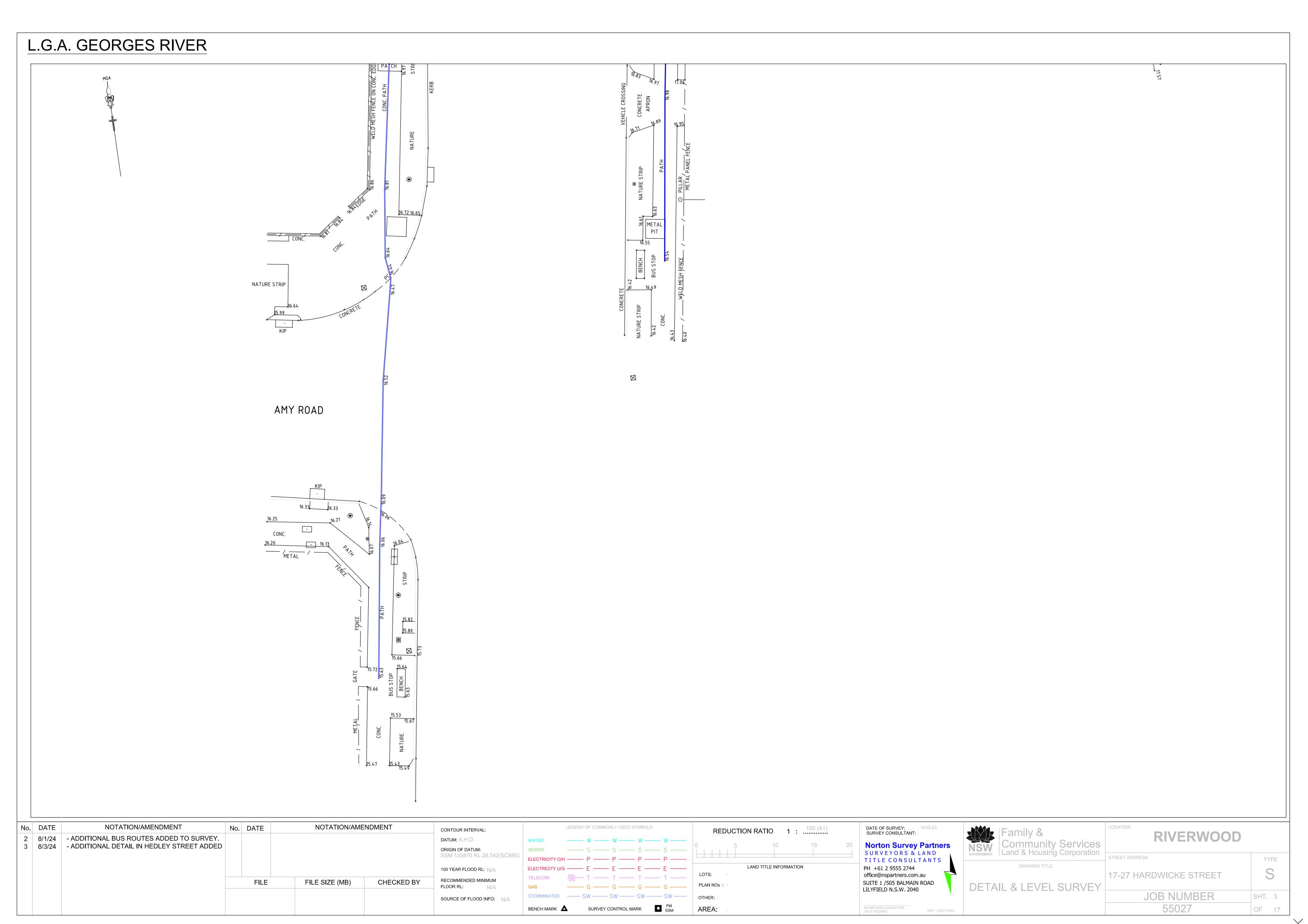
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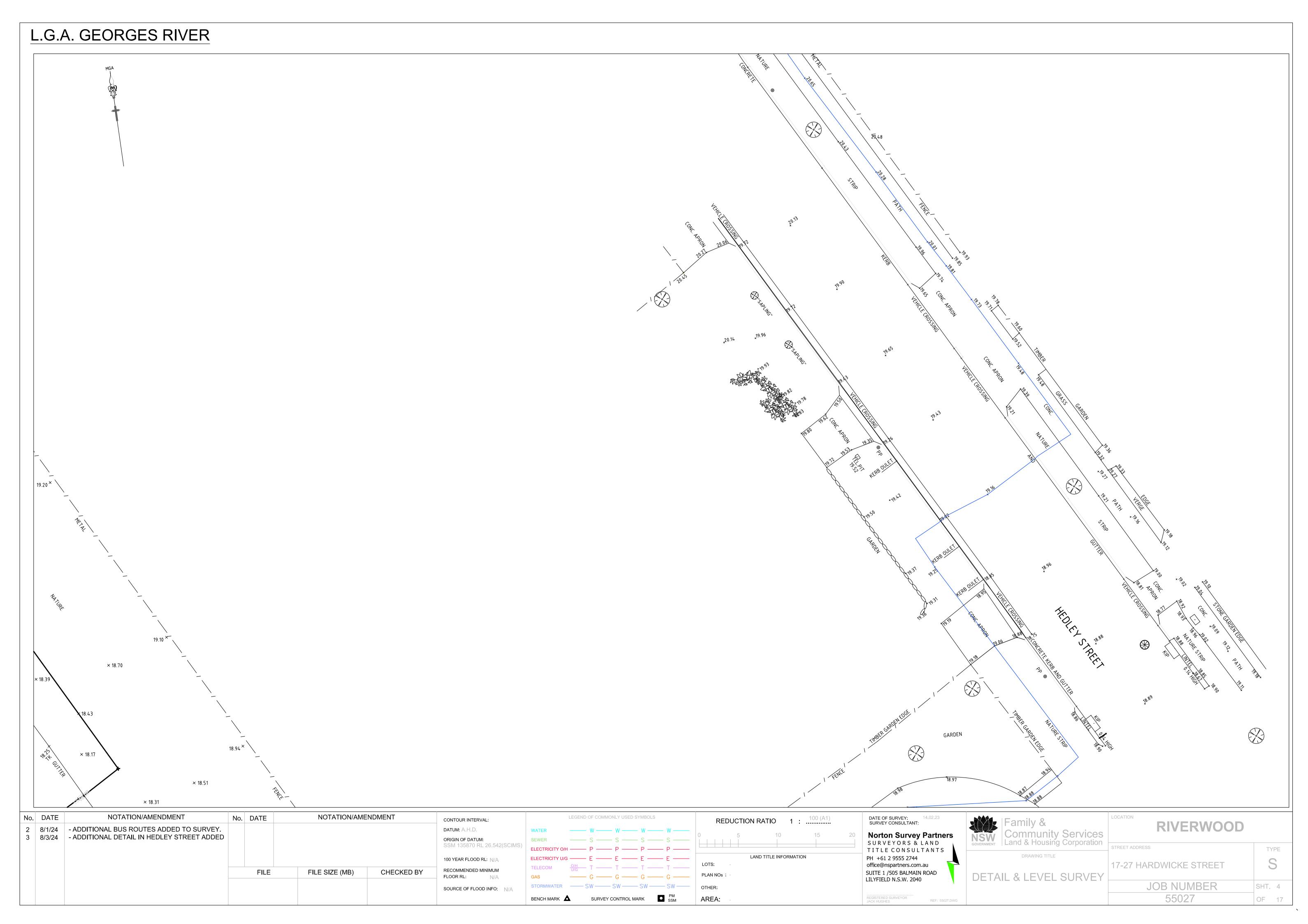


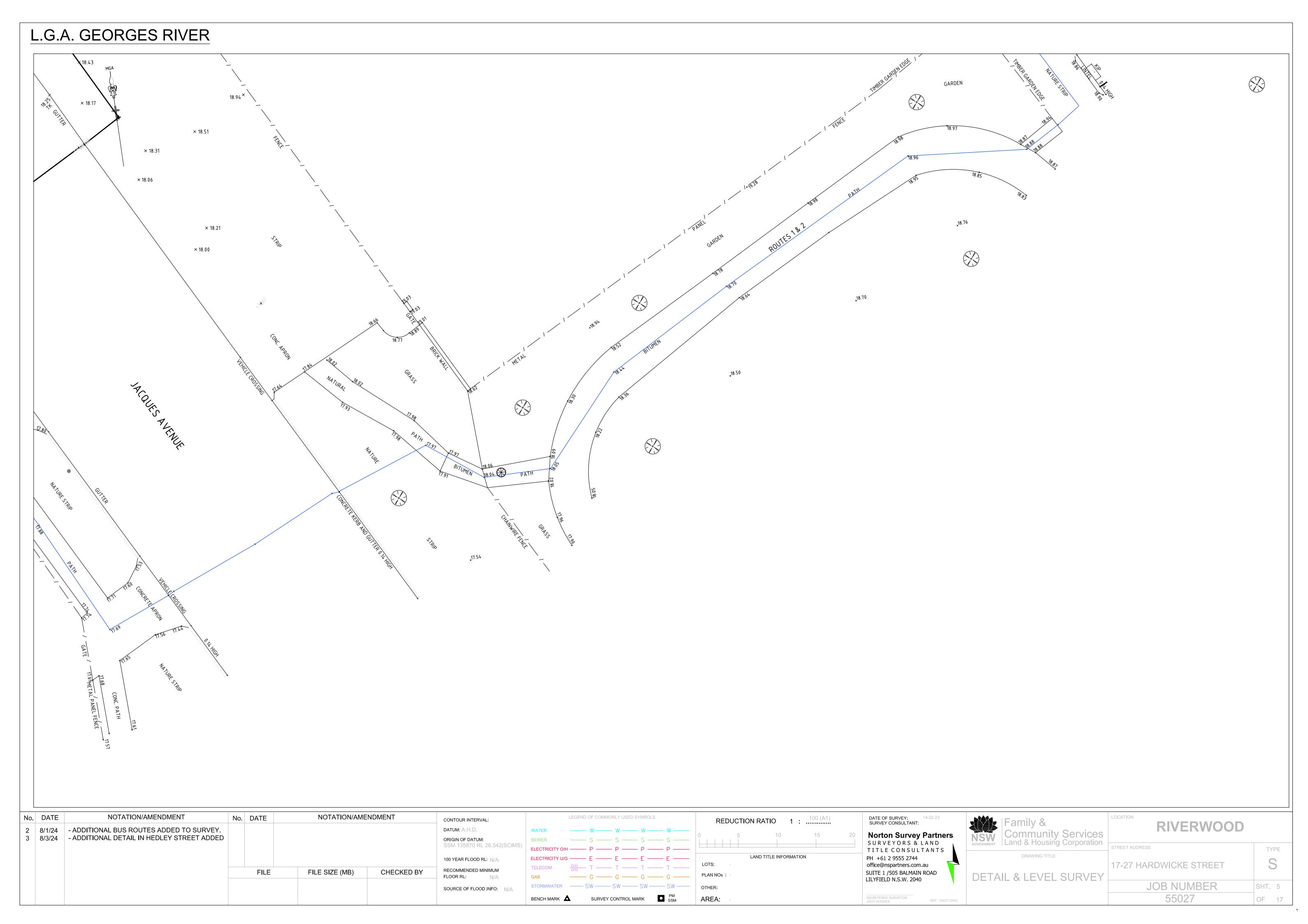
Appendix B: Site Survey

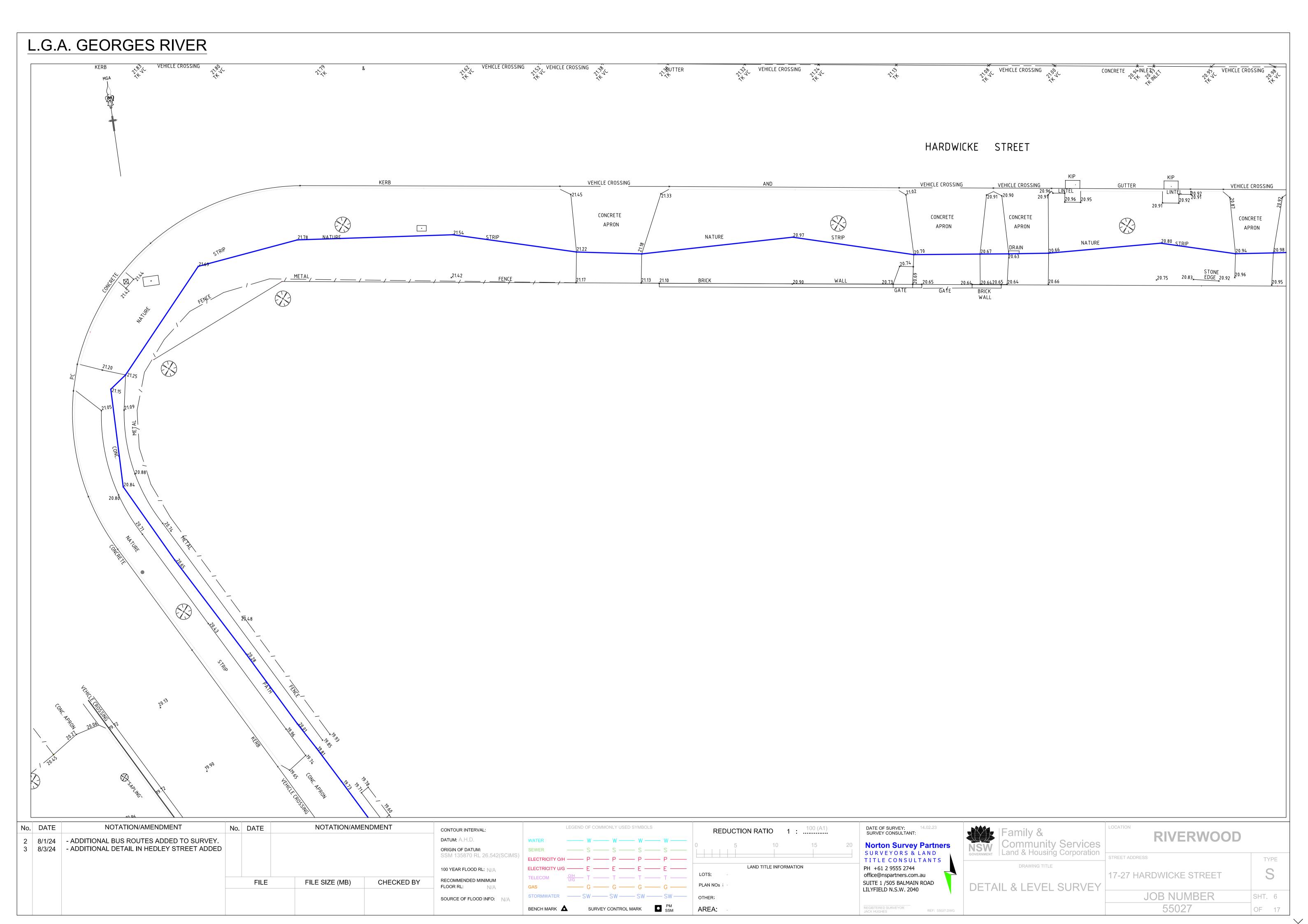


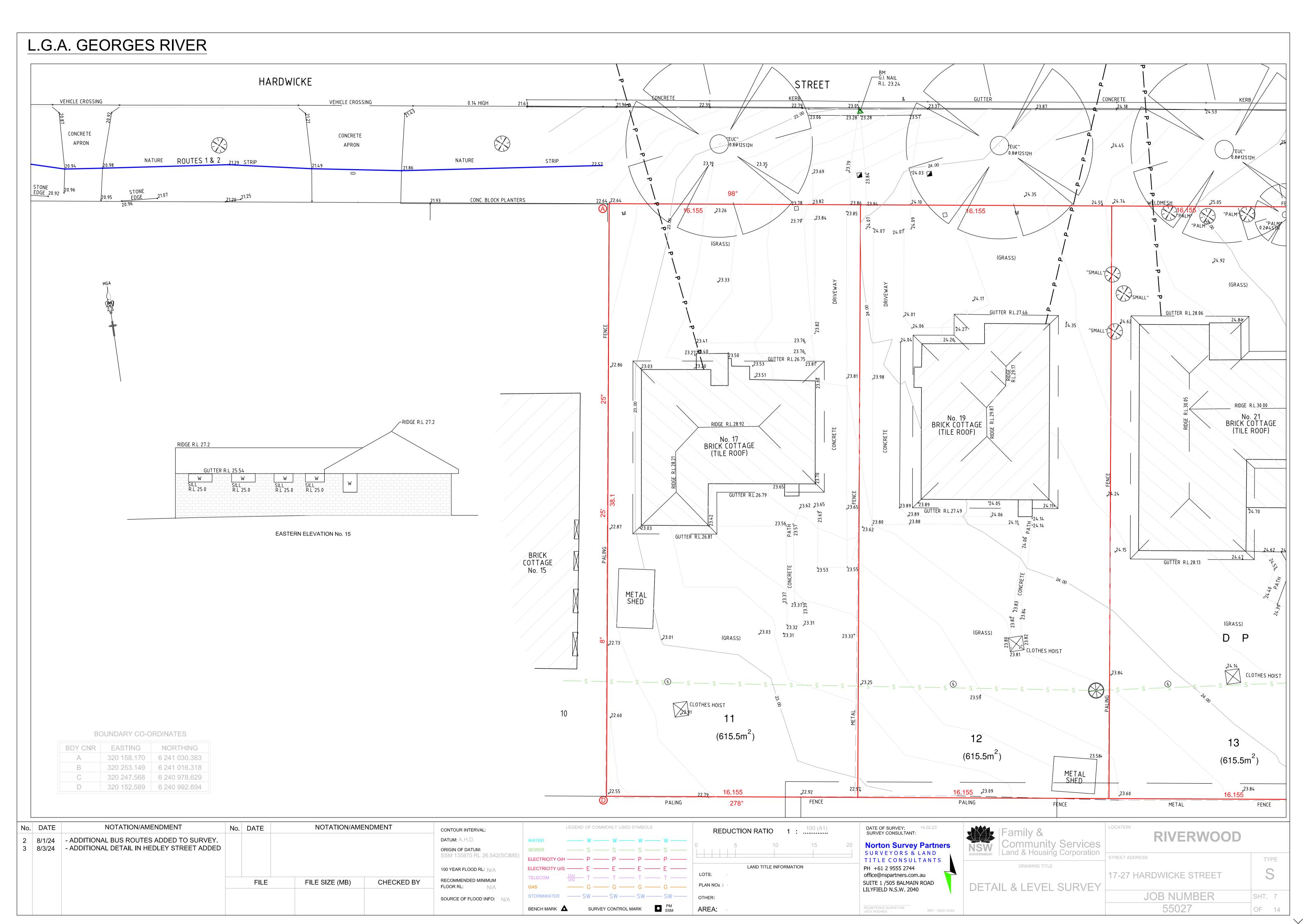


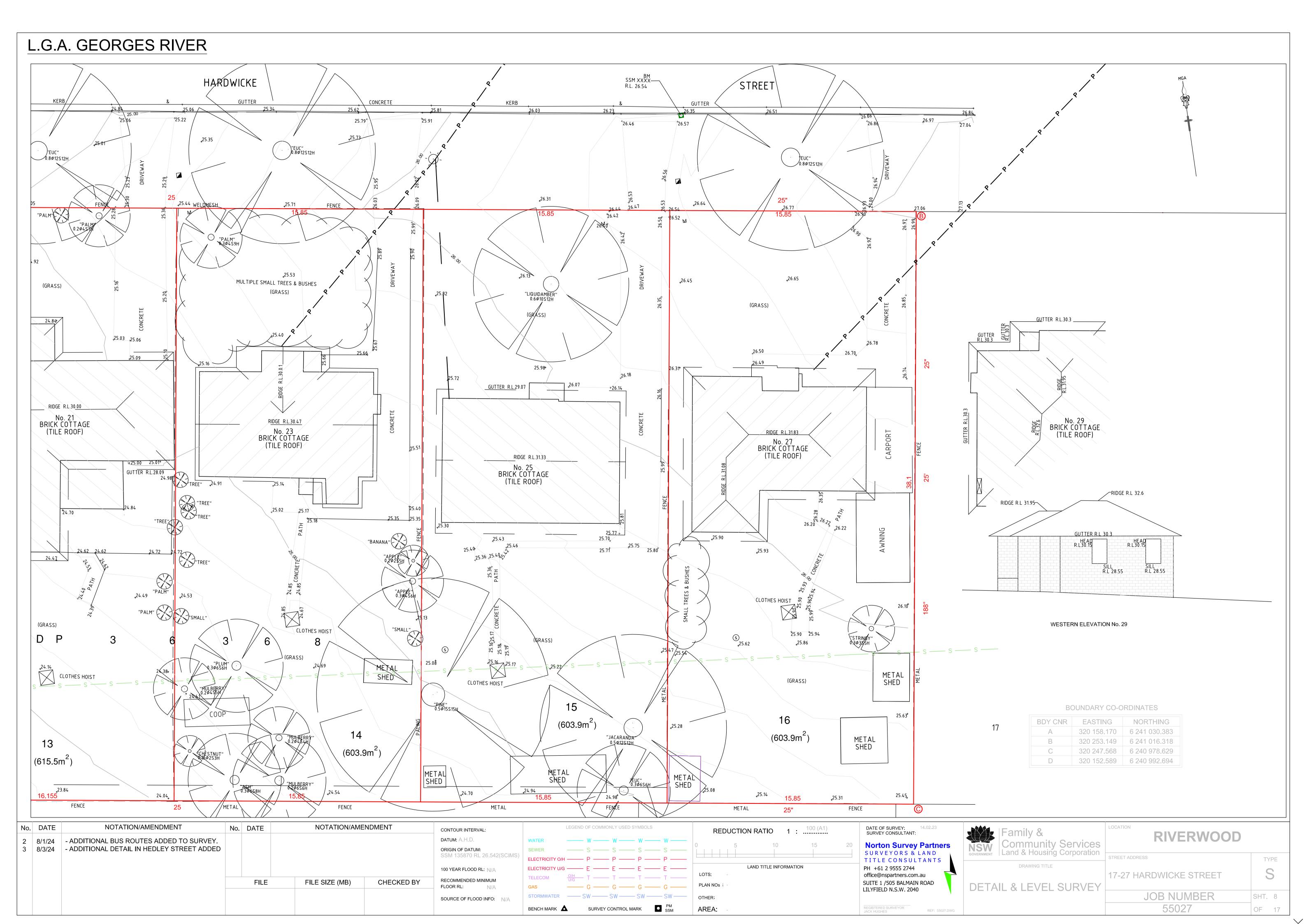




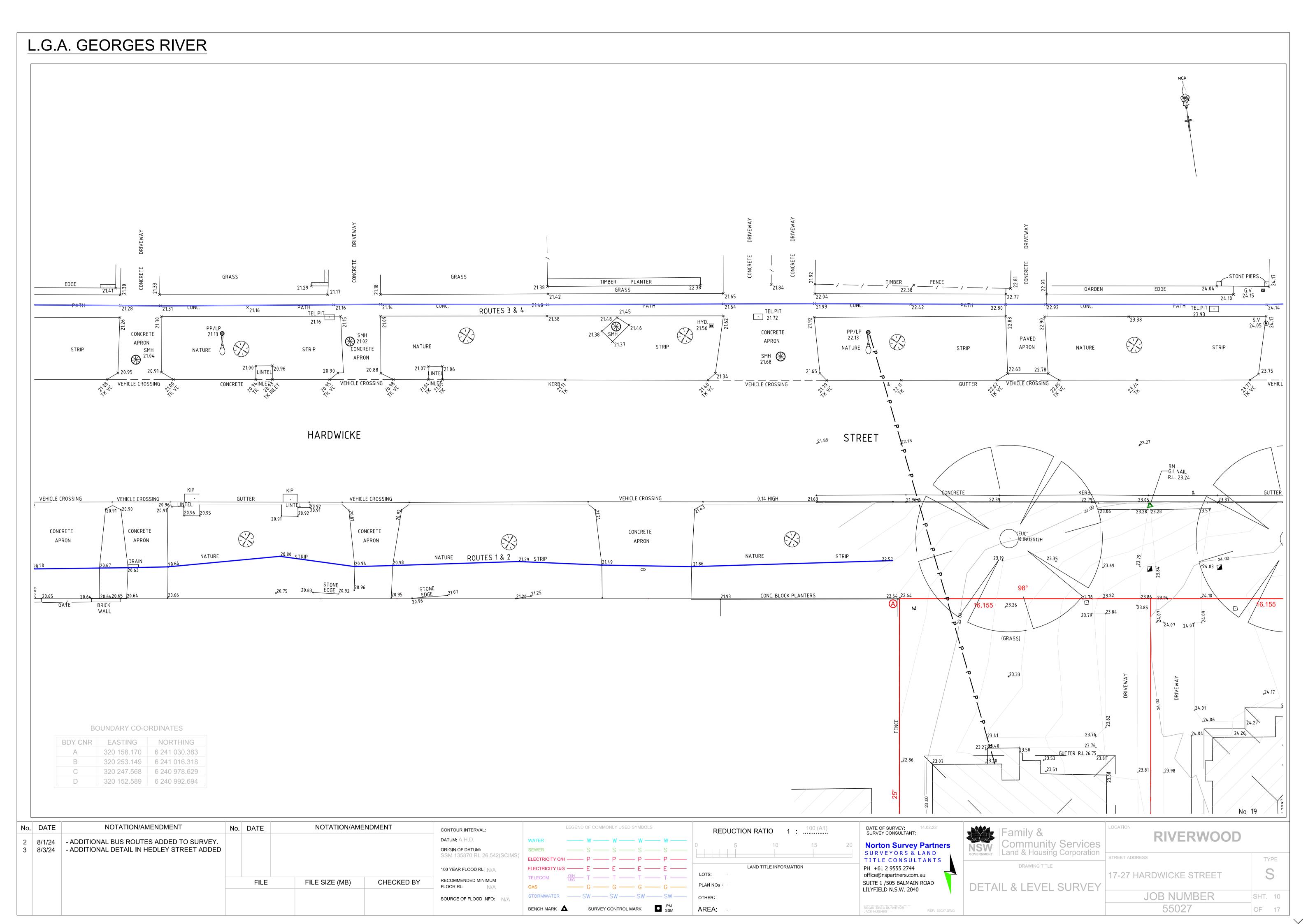


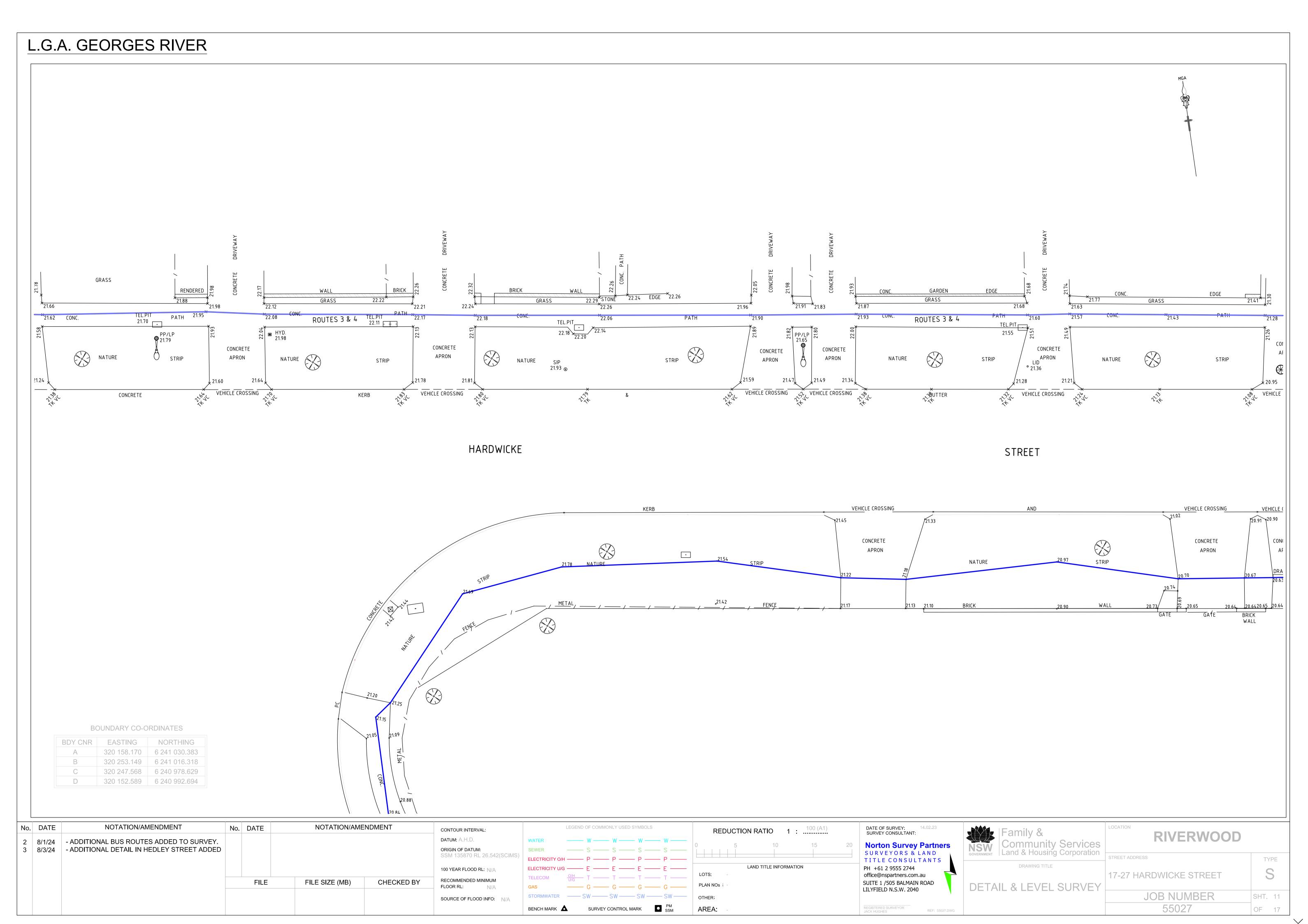




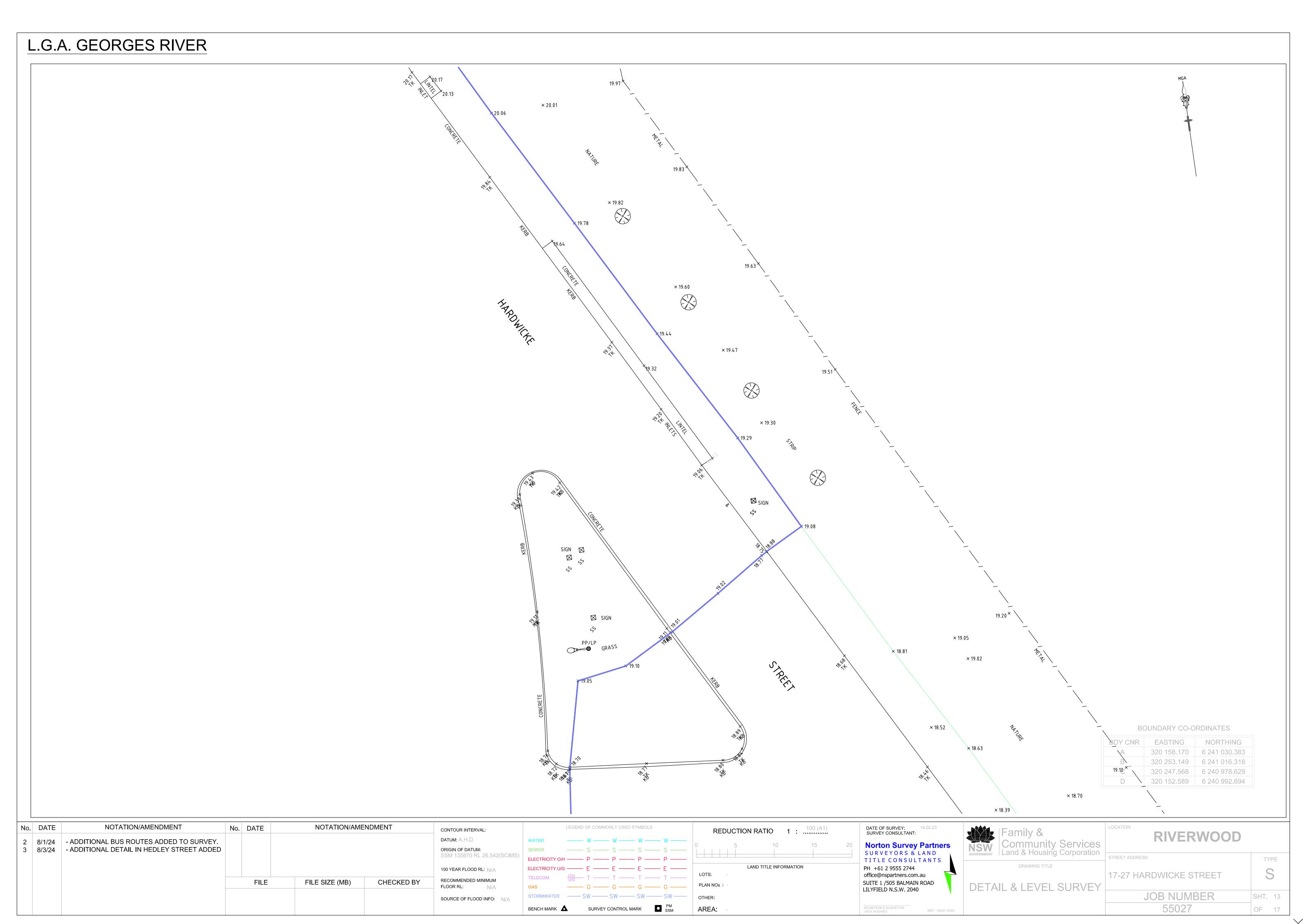


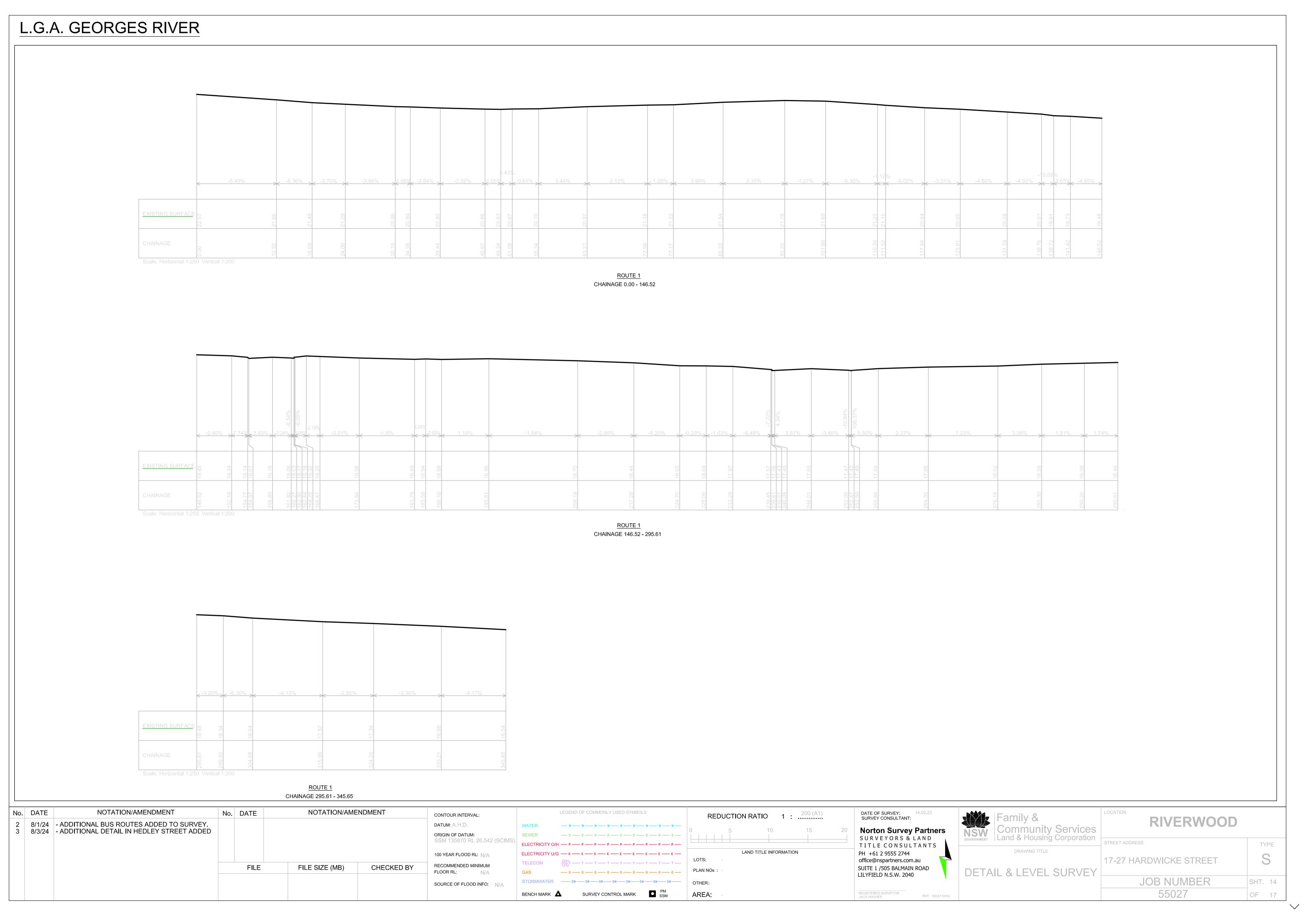
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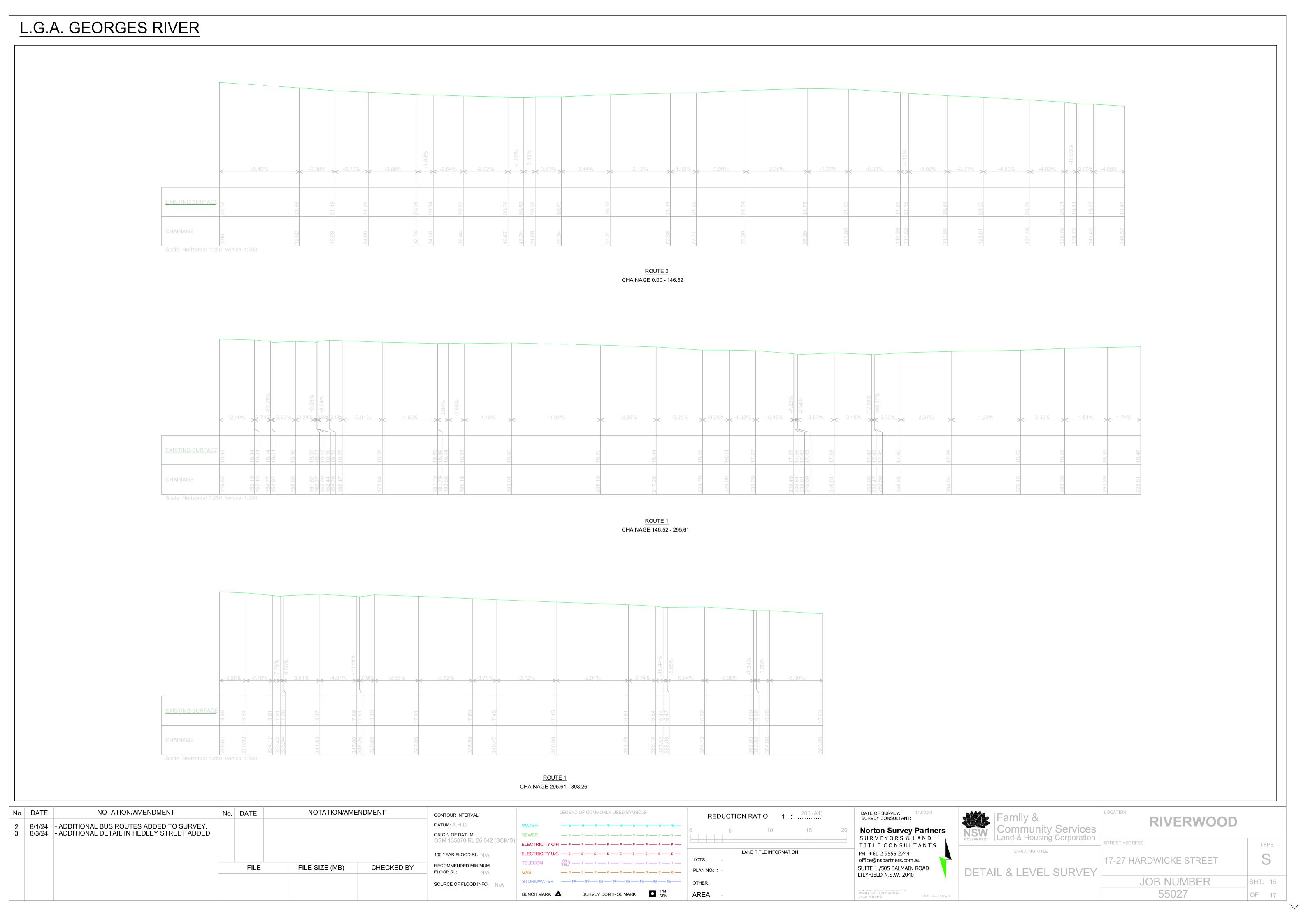




L.G.A. GEORGES RIVER ROUTES 3 & 4 TEL.PIT - 91.72.72.72 TEL.PIT -21.55 TEL.PIT - + - - 21.20 × 21.20 CONCRETE STRIP STRIP × 21.07 APRON LINTEL 20.93 VEHICLE CROSSING CONCRETE SIGN 🛛 × 21.01 HARDWICKE STREET ×20.39 BOUNDARY CO-ORDINATES EASTING NORTHING 320 158.170 6 241 030.383 320 253.149 6 241 016.318 320 247.568 6 240 978.629 320 152.589 6 240 992.694 No. DATE NOTATION/AMENDMENT NOTATION/AMENDMENT LEGEND OF COMMONLY USED SYMBOLS DATE OF SURVEY: 14.02.23 SURVEY CONSULTANT: No. DATE CONTOUR INTERVAL: Family & **RIVERWOOD** DATUM: A.H.D. Community Services Land & Housing Corporation - ADDITIONAL BUS ROUTES ADDED TO SURVEY. Norton Survey Partners SURVEYORS & LAND - ADDITIONAL DETAIL IN HEDLEY STREET ADDED ORIGIN OF DATUM: SSM 135870 RL 26.542(SCIN STREET ADDRESS TITLE CONSULTANTS DRAWING TITLE LAND TITLE INFORMATION PH +61 2 9555 2744 100 YEAR FLOOD RL: N/A 17-27 HARDWICKE STREET office@nspartners.com.au FILE FILE SIZE (MB) RECOMMENDED MINIMUM CHECKED BY SUITE 1 /505 BALMAIN ROAD LILYFIELD N.S.W. 2040 DETAIL & LEVEL SURVEY FLOOR RL: N/A JOB NUMBER SHT. 12 OTHER: SOURCE OF FLOOD INFO: N/A 55027 BENCH MARK SURVEY CONTROL MARK SSM OF 17



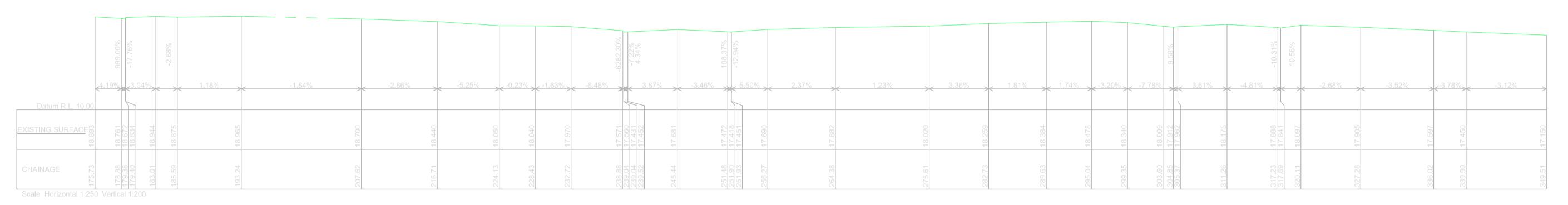




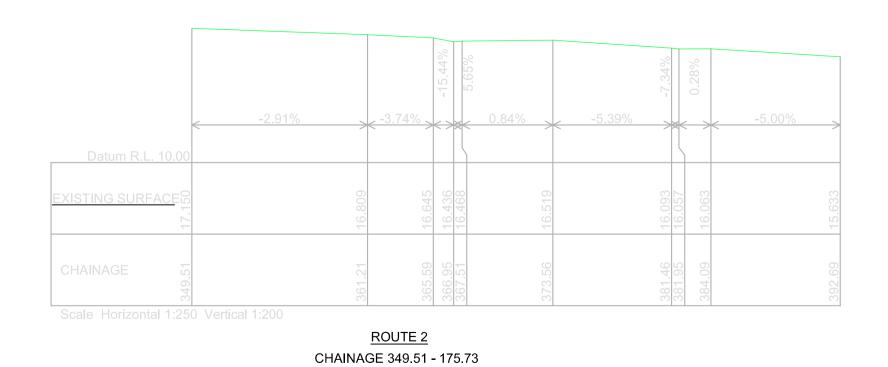
L.G.A. GEORGES RIVER

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EXISTING SURFACE 8912.2		21.859	21.293	20.981	20.802	20.656	20.674	20.700		21.178	, c	7	21.777	200. 200. 200. 200. 200. 200. 200. 200.	21.154	20.646	20.282	20.008	19.810	19.481	19.273	19.023	19.094	18.607
CHAINAGE 00:		12.92	24.06	32.15	39.44	46.67	51.09	55.34		72.98	С	0.7.00	095.20	00.	111.52	123.61	131.19	136.76	138.72	146.52	155.16	163.73	169.41	172.07

ROUTE 2 CHAINAGE 00.00 - 175.73



ROUTE 2 CHAINAGE 175.73 - 349.51



No. DATE NOTATION/AMENDMENT NOTATION/AMENDMENT No. DATE LEGEND OF COMMONLY USED SYMBOLS DATE OF SURVEY: 14.02.23 SURVEY CONSULTANT: Family & CONTOUR INTERVAL: **RIVERWOOD** 8/1/24 - ADDITIONAL BUS ROUTES ADDED TO SURVEY. 8/3/24 - ADDITIONAL DETAIL IN HEDLEY STREET ADDED DATUM: A.H.D. Community Services
Land & Housing Corporation Norton Survey Partners
SURVEYORS & LAND ORIGIN OF DATUM: SSM 135870 RL 26.542 (SCIMS) STREET ADDRESS TYPE TITLE CONSULTANTS LAND TITLE INFORMATION DRAWING TITLE PH +61 2 9555 2744 ELECTRICITY U/G — E — E — E — E — E — E — E — E — E — 100 YEAR FLOOD RL: N/A S 17-27 HARDWICKE STREET office@nspartners.com.au FILE SIZE (MB) RECOMMENDED MINIMUM FILE CHECKED BY SUITE 1 /505 BALMAIN ROAD LILYFIELD N.S.W. 2040 DETAIL & LEVEL SURVEY FLOOR RL: N/A JOB NUMBER SHT. 15 SOURCE OF FLOOD INFO: N/A 55027 BENCH MARK A SURVEY CONTROL MARK SSM REGISTERED SURVEYOR JACK HUGHES OF 17

REF: 55027.DWG

L.G.A. GEORGES RIVER <u>ROUTE 4</u> CHAINAGE 00.00 - 178.09 ROUTE 4 CHAINAGE 178.09 - 362.95 ROUTE 4 CHAINAGE 362.95 - 508.07 No. DATE NOTATION/AMENDMENT NOTATION/AMENDMENT No. DATE LEGEND OF COMMONLY USED SYMBOLS DATE OF SURVEY: 14.02.23 SURVEY CONSULTANT: CONTOUR INTERVAL: Family & **RIVERWOOD** Community Services Land & Housing Corporation 2 8/1/24 - ADDITIONAL BUS ROUTES ADDED TO SURVEY. 3 8/3/24 - ADDITIONAL DETAIL IN HEDLEY STREET ADDED Norton Survey Partners SURVEYORS & LAND ORIGIN OF DATUM: SSM 135870 RL 26.542 (SCIMS) STREET ADDRESS TYPE TITLE CONSULTANTS LAND TITLE INFORMATION DRAWING TITLE PH +61 2 9555 2744 100 YEAR FLOOD RL: N/A S 17-27 HARDWICKE STREET office@nspartners.com.au FILE FILE SIZE (MB) RECOMMENDED MINIMUM CHECKED BY SUITE 1 /505 BALMAIN ROAD LILYFIELD N.S.W. 2040

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BENCH MARK A SURVEY CONTROL MARK SSM

FLOOR RL:

SOURCE OF FLOOD INFO: N/A

DETAIL & LEVEL SURVEY

REGISTERED SURVEYOR JACK HUGHES

REF: 55027.DWG

JOB NUMBER

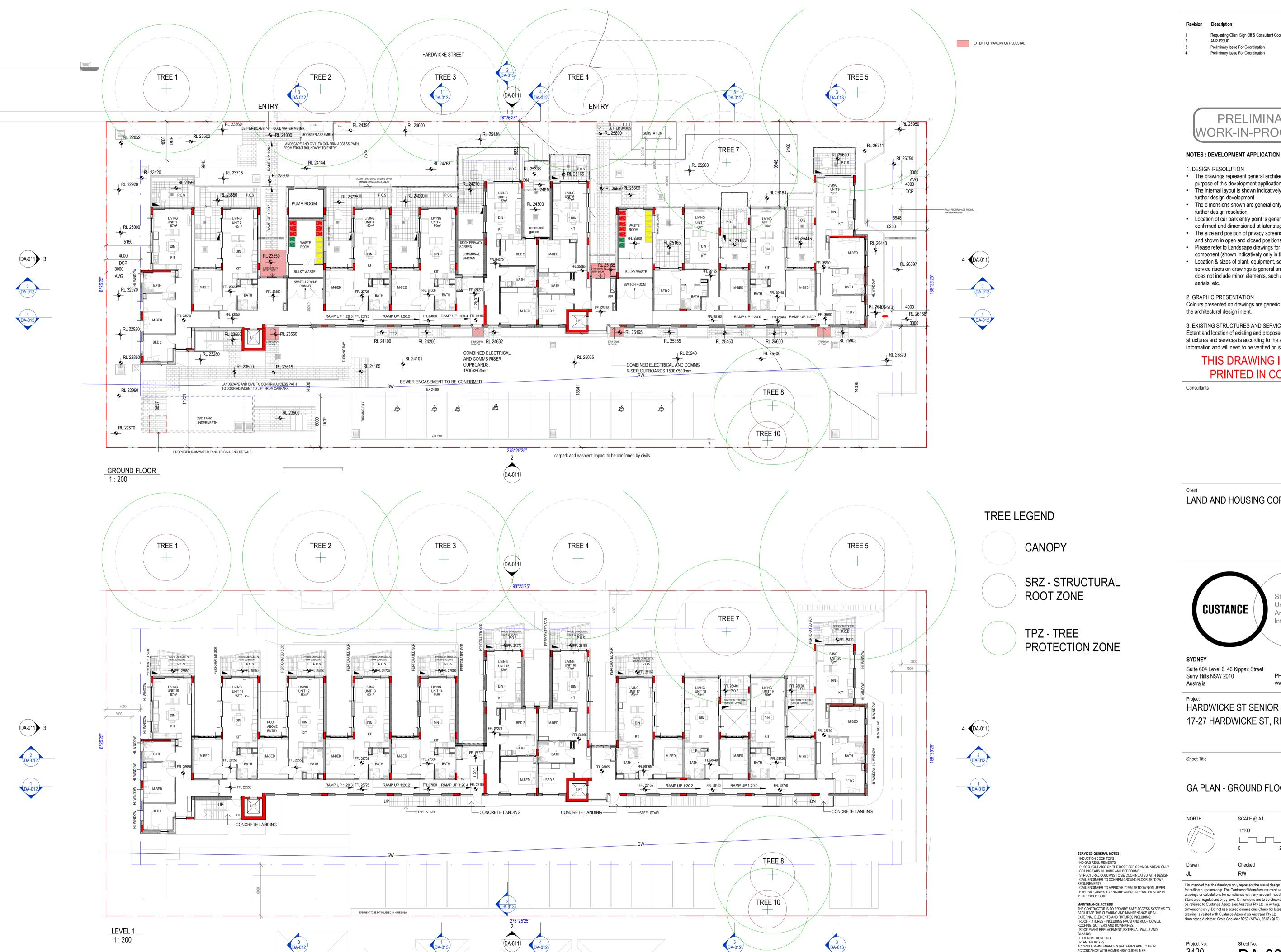
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SHT. 16

OF 17



Appendix C: Architectural concept drawings - Site plan



Revision Description 27.10.2023 Requesting Client Sign Off & Consultant Coordination AM2 ISSUE 15.12.2023 Preliminary Issue For Coordination 30.01.2024 09.02.2024 Preliminary Issue For Coordination

PRELIMINARY WORK-IN-PROGRESS

NOTES: DEVELOPMENT APPLICATION

1. DESIGN RESOLUTION

- The drawings represent general architectural intent for the
- purpose of this development application only. The internal layout is shown indicatively and is subject to
- further design development. The dimensions shown are general only and are subject to
- further design resolution.
- Location of car park entry point is general only and will be
- confirmed and dimensioned at later stage.
- The size and position of privacy screens, louvres is indicative and shown in open and closed positions.
- Please refer to Landscape drawings for Landscape
- component (shown indicatively only in this set)
- Location & sizes of plant, equipment, service areas and service risers on drawings is general and indicative only, and does not include minor elements, such as vent pipes, flues,

2. GRAPHIC PRESENTATION

aerials, etc.

Colours presented on drawings are generic only and indicative of the architectural design intent.

3. EXISTING STRUCTURES AND SERVICES Extent and location of existing and proposed neighbouring structures and services is according to the available survey

information and will need to be verified on site at later stage. THIS DRAWING IS TO BE

PRINTED IN COLOUR Consultants

LAND AND HOUSING CORPORATION



SYDNEY

Suite 604 Level 6, 46 Kippax Street Surry Hills NSW 2010 Australia

PH (02) 9051 0177 www.custance.com.au

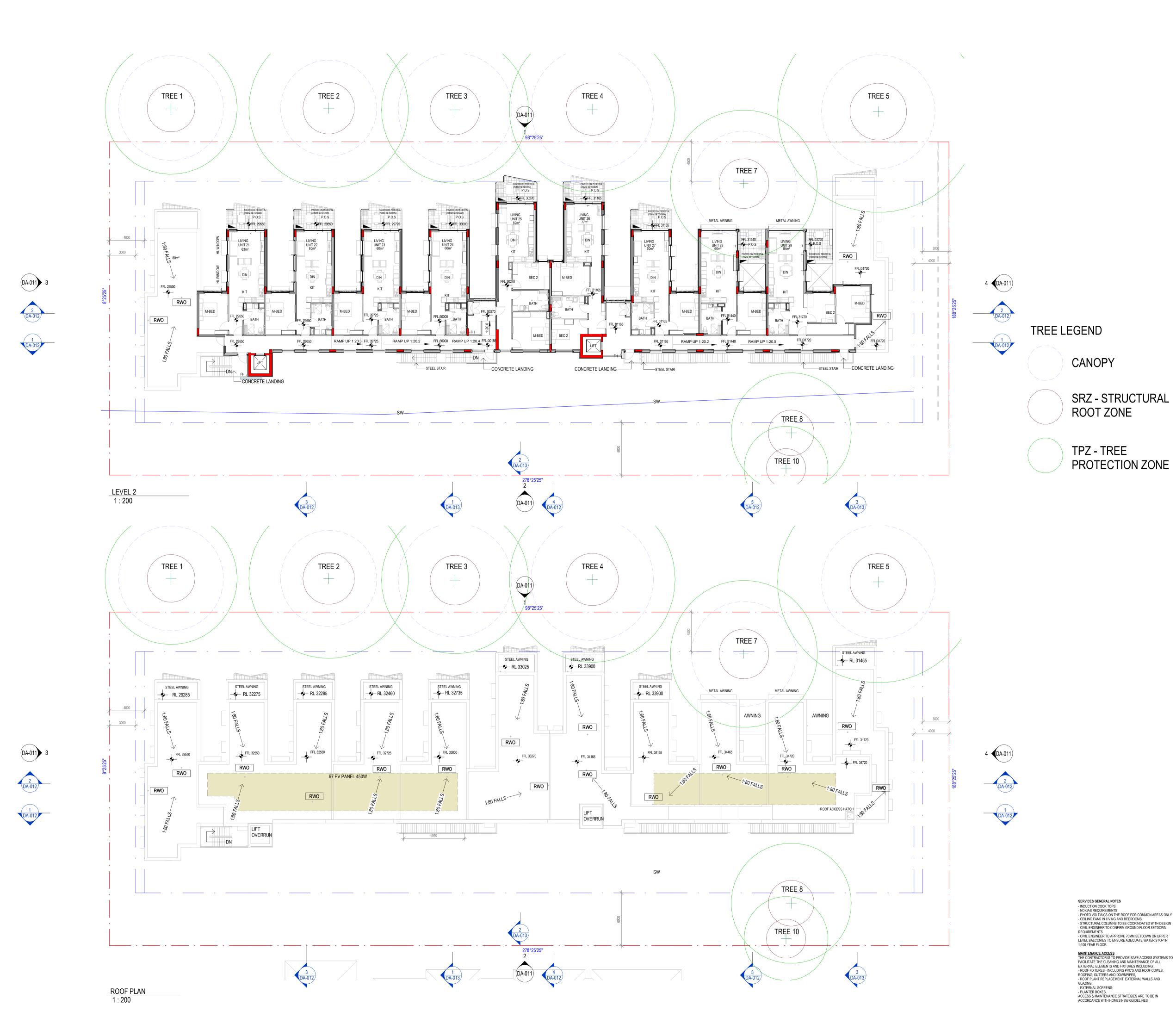
HARDWICKE ST SENIOR HOUSING 17-27 HARDWICKE ST, RIVERWOOD

GA PLAN - GROUND FLOOR & LEVEL 1



RW As indicated @ A1 It is intended that the drawings only represent the visual design of the work. Any technical details are for outline purposes only. The Contractor/ Manufacturer must separately provide all necessary shop drawings or calculations for compliance with any relevant industry, safety standards or Australian Standards, regulations or by-laws. Dimensions are to be checked on site, any discrepancies are to be referred to Custance Associates Australia Pty Ltd, in writing, prior to proceeding. Use written dimensions only. Do not use scaled dimensions. Check for latest revision issue. ©Copyright of this

Project No.



PRELIMINARY WORK-IN-PROGRESS

Requesting Client Sign Off & Consultant Coordination

27.10.2023

15.12.2023

30.01.2024

09.02.2024

NOTES: DEVELOPMENT APPLICATION

AM2 ISSUE

Preliminary Issue For Coordination

Preliminary Issue For Coordination

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 Location & sizes of plant, equipment, service areas and service risers on drawings is general and indicative only, and

does not include minor elements, such as vent pipes, flues,

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Colours presented on drawings are generic only and indicative of the architectural design intent.

3. EXISTING STRUCTURES AND SERVICES Extent and location of existing and proposed neighbouring structures and services is according to the available survey information and will need to be verified on site at later stage.

> THIS DRAWING IS TO BE PRINTED IN COLOUR

Consultants

LAND AND HOUSING CORPORATION



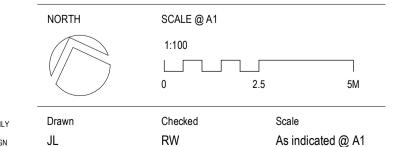
SYDNEY

Suite 604 Level 6, 46 Kippax Street Surry Hills NSW 2010 Australia

PH (02) 9051 0177 www.custance.com.au

HARDWICKE ST SENIOR HOUSING 17-27 HARDWICKE ST, RIVERWOOD

GA PLAN - LEVEL 2 & ROOF PLAN



It is intended that the drawings only represent the visual design of the work. Any technical details are for outline purposes only. The Contractor/ Manufacturer must separately provide all necessary shop drawings or calculations for compliance with any relevant industry, safety standards or Australian Standards, regulations or by-laws. Dimensions are to be checked on site, any discrepancies are to be referred to Custance Associates Australia Pty Ltd, in writing, prior to proceeding. Use written dimensions only. Do not use scaled dimensions. Check for latest revision issue. ©Copyright of this drawing is vested with Custance Associates Australia Pty Ltd Nominated Architect: Craig Shelsher 8259 (NSW), 5612 (QLD)

Project No. **3420**

DA-007



Appendix D: Maintenance manuals



StormFilter

Operations & Maintenance Manual

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Introduction2	
Why do I need to perform maintenance?	2
Health and Safety	
Personnel health and safety	3
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Maintenance Procedures	
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Inspection	
Minor Service	
Major Service (Filter Cartridge Replacement)	
Additional Types of Maintenance	6
Hazardous Material Spill	6
Blockages	6
Major Storms and Flooding	6
Disposal of Waste Materials	6
Maintenance Services	

Introduction

The primary purpose of stormwater treatment devices is to capture and prevent pollutants from entering waterways, maintenance is a critical component of ensuring the ongoing effectiveness of this process. The specific requirements and frequency for maintenance depends on the treatment device and pollutant load characteristics of each site. This manual has been designed to provide details on the cleaning and maintenance processes for the StormFilter as recommended by the manufacturer.

The StormFilter is designed and sized to meet stringent regulatory requirements. It removes the most challenging target pollutants (including fine solids, soluble heavy metals, oil, and soluble nutrients) using a variety of media. For more than two decades, StormFilter has helped clients meet their regulatory needs and, through ongoing product enhancements, the design continues to be refined for ease of use and improved performance.

Why do I need to perform maintenance?

Adhering to the inspection and maintenance schedule of each stormwater treatment device is essential to ensuring that it functions properly throughout its design life.

During each inspection and clean, details of the mass, volume and type of material that has been collected by the device should be recorded. This data will assist with the revision of future management plans and help determine maintenance interval frequency. It is also essential that qualified and experienced personnel carry out all maintenance (including inspections, recording and reporting) in a systematic manner.

Maintenance of your stormwater management system is essential to ensuring ongoing at-source control of stormwater pollution. Maintenance also helps prevent structural failures (e.g. prevents blocked outlets) and aesthetic failures (e.g. debris build up), but most of all ensures the long term effective operation of the StormFilter.

Health and Safety

Access to a StormFilter unit requires removing heavy access covers/grates, and it is necessary to enter into a confined space. Pollutants collected by the StormFilter will vary depending on the nature of your site. There is potential for these materials to be harmful. For example, sediments may contain heavy metals, carcinogenic substances or objects such as broken glass and syringes. For these reasons, all aspects of maintaining and cleaning your StormFilter require careful adherence to Occupational Health and Safety (OH&S) guidelines.

It is important to note that the same level of care needs to be taken to ensure the safety of non-work personnel. As a result, it may be necessary to employ traffic/pedestrian control measures when the device is situated in, or near areas with high vehicular/pedestrian activity.

Personnel health and safety

Whilst performing maintenance on the StormFilter, precautions should be taken in order to minimise (or, if possible, prevent) contact with sediment and other captured pollutants by maintenance personnel. The following personal protective equipment (PPE) is subsequently recommended:

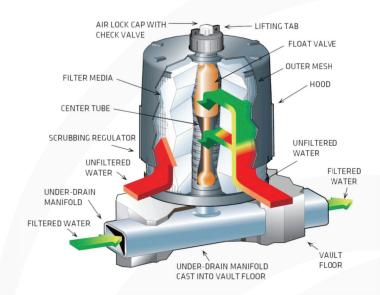
- Puncture resistant gloves
- Steel capped safety boots
- Long sleeve clothing, overalls or similar skin protection
- Eye protection
- High visibility clothing or vest

During maintenance activities, it may be necessary to implement traffic control measures. Ocean Protect recommend that a separate site-specific traffic control plan is implemented as required to meet the relevant governing authority guidelines.

Whilst some aspects of StormFilter maintenance can be performed from surface level, there will be a need to enter the StormFilter system (confined space) during a major service. It is recommended that all maintenance personnel evaluate their own needs for confined space entry and compliance with relevant industry regulations and guidelines. Ocean Protect maintenance personnel are fully trained and carry certification for confined space entry applications.

How does it Work?

Stormwater enters the cartridge chamber, passes through the filtration media and begins filling the cartridge center tube. When water reaches the top of the cartridge the float valve opens and filtered water is allowed to drain at the designed flow rate. Simultaneously, a one-way check valve closes activating a siphon that draws stormwater evenly throughout the filter media and into the center tube. Treated stormwater is then able to discharge out of the system through the underdrain manifold pipework.



As the rain event subsides, the water level outside the cartridge drops and approaches the bottom of the hood, air rushes through the scrubbing regulators releasing the water column and breaking the siphon. The turbulent bubbling action agitates the surface of the cartridge promoting trapped sediment to drop to the chamber floor. After a rain event, the chamber is able to drain dry by way of an imperfect seal at the base of the float valve.

Maintenance Procedures

To ensure optimal performance, it is advisable that regular maintenance is performed. Typically, the StormFilter requires an inspection every 6 months with a minor service at 12 months. Additionally, as the StormFilter cartridges capture pollutants the media will eventually become occluded and require replacement (expected media life is 1-3 years).

Primary Types of Maintenance

The table below outlines the primary types of maintenance activities that typically take place as part of an ongoing maintenance schedule for the StormFilter.

	Description of Typical Activities	Frequency
Inspection	Visual Inspection of cartridges & chamber Remove larger gross pollutants Perform minimal rectification works (if required)	Every 6 Months
Minor Service	Evaluation of cartridges and media Removal of accumulated sediment (if required) Wash-down of StormFilter chamber (if required)	Every 12 Months
Major Service	Replacement of StormFilter cartridge media	As required

Ocean Protect | StormFilter Operations & Maintenance Manual

Maintenance requirements and frequencies are dependent on the pollutant load characteristics of each site. The frequencies provided in this document represent what the manufacturer considers to be best practice to ensure the continuing operation of the device is in line with the original design specification.

Inspection

The purpose of the inspecting the StormFilter system is to assess the condition of the StormFilter chamber and cartridges. When inspecting the chamber, particular attention should be taken to ensure all cartridges are firmly connected to the connectors. It is also an optimal opportunity to remove larger gross pollutants and inspect the outlet side of the StormFilter weir.

Minor Service

This service is designed to ensure the ongoing operational effectiveness of the StormFilter system, whilst assessing the condition of the cartridge media.

- 1. Establish a safe working area around the access point(s)
- 2. Remove access cover(s)
- 3. Evaluate StormFilter cartridge media (if exhausted schedule major service within 6 months)
- 4. Measure and record the level of accumulated sediment in the chamber (if sediment depth is less than 100 mm skip to step 9)
- 5. Remove StormFilter cartridges from the chamber
- 6. Use vacuum unit to removed accumulated sediment and pollutants in the chamber
- 7. Use high pressure water to clean StormFilter chamber
- 8. Re-install StormFilter cartridges
- 9. Replace access cover(s)

Major Service (Filter Cartridge Replacement)

For the StormFilter system a major service is reactionary process based on the outcomes from the minor service, specifically the evaluation of the cartridge media.

Trigger Event	Maintenance Action
Cartridge media is exhausted ^[1]	Replace StormFilter cartridge media ^[2]

[1] Multiple assessment methods are available, contact Ocean Protect for assistance

[2] Replacement filter media and components are available for purchase from Ocean Protect.

This service is designed to return the StormFilter device back to optimal operating performance

- 1. Establish a safe working area around the access point(s)
- 2. Remove access cover(s)
- 3. By first removing the head cap, remove each individual cartridge hood to allow access to the exhausted media.
- 4. Utilise a vacuum unit to remove exhausted media from each cartridge
- 5. Use vacuum unit to remove accumulated sediment and pollutants in the chamber
- 6. Use high pressure water to clean StormFilter chamber
- 7. Inspect each empty StormFilter cartridges for any damage, rectify damage as required
- 8. Re-fill each cartridge with media in line with project specifications
- 9. Re-install replenished StormFilter cartridges
- 10. Replace access cover(s)

Additional Types of Maintenance

Occasionally, events on site can make it necessary to perform additional maintenance to ensure the continuing performance of the device.

Hazardous Material Spill

If there is a spill event on site, the StormFilter unit should be inspected and cleaned. Specifically, all captured pollutants and liquids from within the unit should be removed and disposed in accordance with any additional requirements that may relate to the type of spill event. Additionally, it will be necessary to inspect the filter cartridges and assess them for contamination, depending on the type of spill event it may be necessary to replace the filtration media.

Blockages

In the unlikely event that flooding occurs upstream of the StormFilter system the following steps should be undertaken to assist in diagnosing the issue and determining the appropriate response.

- 1. Inspect the upstream diversion structure (if applicable) ensuring that it is free of debris and pollutants
- 2. Inspect the StormFilter unit checking the underdrain manifold as well as both the inlet and outlet pipes for obstructions (e.g. pollutant build-up, blockage), which if present, should be removed.

Major Storms and Flooding

In addition to the scheduled activities, it is important to inspect the condition of the StormFilter after a major storm event. The focus is to inspect for damage and higher than normal sediment accumulation that may result from localised erosion. Where necessary damaged components should be replaced and accumulated pollutants should be removed and disposed.

Disposal of Waste Materials

The accumulated pollutants found in the StormFilter must be handled and disposed of in a manner that is in accordance with all applicable waste disposal regulations. When scheduling maintenance, consideration must be made for the disposal of solid and liquid wastes. If the filter media has been contaminated with any unusual substance, there may be additional special handling and disposal methods required to comply with relevant government/authority/industry regulations.

Maintenance Services

With over a decade and a half of maintenance experience Ocean Protect has developed a systematic approach to inspecting, cleaning and maintaining a wide variety of stormwater treatment devices. Our fully trained and professional staff are familiar with the characteristics of each type of system, and the processes required to ensure its optimal performance.

Ocean Protect has several stormwater maintenance service options available to help ensure that your stormwater device functions properly throughout its design life. In the case of our StormFilter system we offer long term pay-as-you-go contracts, pre-paid once off servicing and replacement media for cartridges.

For more information please visit www.OceanProtect.com.au



OceanGuard™

Operations & Maintenance Manual

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Introduction

The primary purpose of stormwater treatment devices is to capture and prevent pollutants from entering waterways, maintenance is a critical component of ensuring the ongoing effectiveness of this process. The specific requirements and frequency for maintenance depends on the treatment device and pollutant load characteristics of each site. This manual has been designed to provide details on the cleaning and maintenance processes as recommended by the manufacturer.

The OceanGuard technology is a gully pit basket designed to fit within new and existing gully pits to remove pollution from stormwater runoff. The system has a choice of Filtration liners, designed to remove gross pollutants, total suspended solids and attached pollutants as either a standalone technology or as part of a treatment train with our StormFilter or Jellyfish Filtration products. OceanGuard pit baskets are highly effective, easy to install and simple to maintain.

Why do I need to perform maintenance?

Adhering to the maintenance schedule of each stormwater treatment device is essential to ensuring that it functions properly throughout its design life.

During each inspection and clean, details of the mass, volume and type of material that has been collected by the device should be recorded. This data will assist with the revision of future management plans and help determine maintenance interval frequency. It is also essential that qualified and experienced personnel carry out all maintenance (including inspections, recording and reporting) in a systematic manner.

Maintenance of your stormwater management system is essential to ensuring ongoing at-source control of stormwater pollution. Maintenance also helps prevent structural failures (e.g. prevents blocked outlets) and aesthetic failures (e.g. debris build up), but most of all ensures the long term effective operation of the OceanGuard.

Health and Safety

Access to pits containing an OceanGuard typically requires removing (heavy) access covers/grates, but typically it is not necessary to enter into a confined space. Pollutants collected by the OceanGuard will vary depending on the nature of your site. There is potential for these materials to be harmful. For example, sediments may contain heavy metals, carcinogenic substances or sharp objects such as broken glass and syringes. For these reasons, there should be no primary contact with the waste collect and all aspects of maintaining and cleaning your OceanGuard require careful adherence to Occupational Health and Safety (OH&S) guidelines.

It is important to note that the same level of care needs to be taken to ensure the safety of non-work personnel, as a result it may be necessary to employ traffic/pedestrian control measures when the device is situated in, or near areas with high vehicular/pedestrian activity.

Personnel health and safety

Whilst performing maintenance on the OceanGuard pit insert, precautions should be taken in order to minimise (or when possible prevent) contact with sediment and other captured pollutants by maintenance personnel. In order to achieve this the following personal protective equipment (PPE) is recommended:

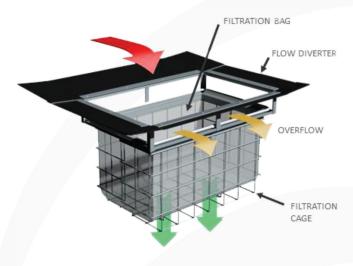
- Puncture resistant gloves
- Steel capped safety boots,
- Long sleeve clothing, overalls or similar skin protection
- Eye protection
- High visibility clothing or vest

During maintenance activities it may be necessary to implement traffic control measures. Ocean Protect recommend that a separate site specific traffic control plan is implemented as required to meet the relevant governing authority guidelines.

The OceanGuard pit insert is designed to be maintained from surface level, without the need to enter the pit. However depending on the installation configuration, location and site specific maintenance requirements it may be necessary to enter a confined space occasionally. It is recommended that all maintenance personnel evaluate their own needs for confined space entry and compliance with relevant industry regulations and guidelines. Ocean Protect maintenance personnel are fully trained and carry certification for confined space entry.

How does it Work?

OceanGuard is designed to intercept stormwater as it enters the stormwater pits throughout a site. The OceanGuard has diversion panels that sit flush with the pit walls, this ensures that as stormwater enters at the top of the pit it is directed to the middle of the insert where the Filtration bag is situated. The filtration bag allows for screening to occur removing 100% of pollutants greater than the opening of the filtration material (200micron, 1600micron bags available).



During larger rain events the large flows overflow slots in the flow diverter of the OceanGuard ensure that the conveyance of stormwater is not impeded thus eliminating the potential for surface flooding. As the flow subsides, the captured pollutants are held in the OceanGuard Filtration bag dry. The waste then starts to dry which reduces the magnitude of organic material decomposition transitioning between maintenance intervals.

Maintenance Procedures

To ensure that each OceanGuard pit insert achieves optimal performance, it is advisable that regular maintenance is performed. Typically the OceanGuard requires 2-4 minor services annually, pending the outcome of these inspections additional maintenance servicing may be required.

Primary Types of Maintenance

The table below outlines the primary types of maintenance activities that typically take place as part of an ongoing maintenance schedule for the OceanGuard.

	Description of Typical Activities	Frequency
Minor Service	Filter bag inspection and evaluation Removal of capture pollutants Disposal of material	2-4 Times Annually
Major Service	Filter Bag Replacement Support frame rectification	As required

Ocean Protect | OceanGuard Operations & Maintenance Manual

Maintenance requirements and frequencies are dependent on the pollutant load characteristics of each site. The frequencies provided in this document represent what the manufacturer considers to be best practice to ensure the continuing operation of the device is in line with the original design specification.

Minor Service

This service is designed to return the OceanGuard device back to optimal operating performance. This type of service can be undertaken either by hand or with the assistance of a Vacuum unit.

Hand Maintenance

- 1. Establish a safe working area around the pit insert
- 2. Remove access cover/grate
- 3. Use two lifting hooks to remove the filtration bag
- 4. Empty the contents of the filtration bag into a disposal container
- 5. Inspect and evaluate the filtration bag
- 6. Inspect and evaluate remaining OceanGuard components (i.e. flow diverter, filtration cage and supporting frame)
- 7. Rejuvenate filtration bag by removing pollutant build up with a stiff brush, additionally the filtration bag can be washed using high pressure water
- 8. Re-install filtration bag and replace access cover/grate

Vacuum Maintenance

- 1. Establish a safe working area around the pit insert
- 2. Remove access cover/grate
- 3. Vacuum captured pollutants from the filtration bag
- 4. Remove filtration bag
- 5. Inspect and evaluate the filtration bag
- 6. Inspect and evaluate remaining OceanGuard components (i.e. flow diverter, filtration cage and supporting frame)
- 7. Rejuvenate filtration bag by removing pollutant build up with a stiff brush, additionally the filtration bag can be washed using high pressure water
- 8. Re-install filtration bag and replace access cover/grate

Major Service (Filter Bag Replacement)

For the OceanGuard system, a major service is a reactionary process based on the outcomes from the minor service.

Trigger Event from Minor Service	Maintenance Action
Filtration bag inspection reveals damage	Replace the filtration bag ^[1]
Component inspection reveals damage	Perform rectification works and if necessary replace components ^[1]

[1] Replacement filtration bags and components are available for purchase from Ocean Protect.

Additional Reasons of Maintenance

Occasionally, events on site can make it necessary to perform additional maintenance to ensure the continuing performance of the device.

Hazardous Material Spill

If there is a spill event on site, all OceanGuard pits that potentially received flow should be inspected and cleaned. Specifically all captured pollutants from within the filtration bag should be removed and disposed in accordance with any additional requirements that may relate to the type of spill event. All filtration bags should be rejuvenated (replaced if required) and re-installed.

Blockages

The OceanGuards internal high flow bypass functionality is designed to minimise the potential of blockages/flooding. In the unlikely event that flooding occurs around the stormwater pit the following steps should be undertaken to assist in diagnosing the issue and implementing the appropriate response.

- 1. Inspect the OceanGuard flow diverter, ensuring that they are free of debris and pollutants
- 2. Perform a minor service on the OceanGuard
- 3. Remove the OceanGuard insert to access the pit and inspect both the inlet and outlet pipes, ensuring they are free of debris and pollutants

Major Storms and Flooding

In addition to the scheduled activities, it is important to inspect the condition of the OceanGuard pit insert after a major storm event. The inspection should focus on checking for damage and higher than normal sediment accumulation that may result from localised erosion. Where necessary damaged components should be replaced and accumulated pollutants disposed.

Disposal of Waste Materials

The accumulated pollutants found in the OceanGuard must be handled and disposed of in a manner that is in accordance with all applicable waste disposal regulations. When scheduling maintenance, consideration must be made for the disposal of solid and liquid wastes. If the filtration bag has been contaminated with any unusual substance, there may be additional special handling and disposal methods required to comply with relevant government/authority/industry regulations.

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